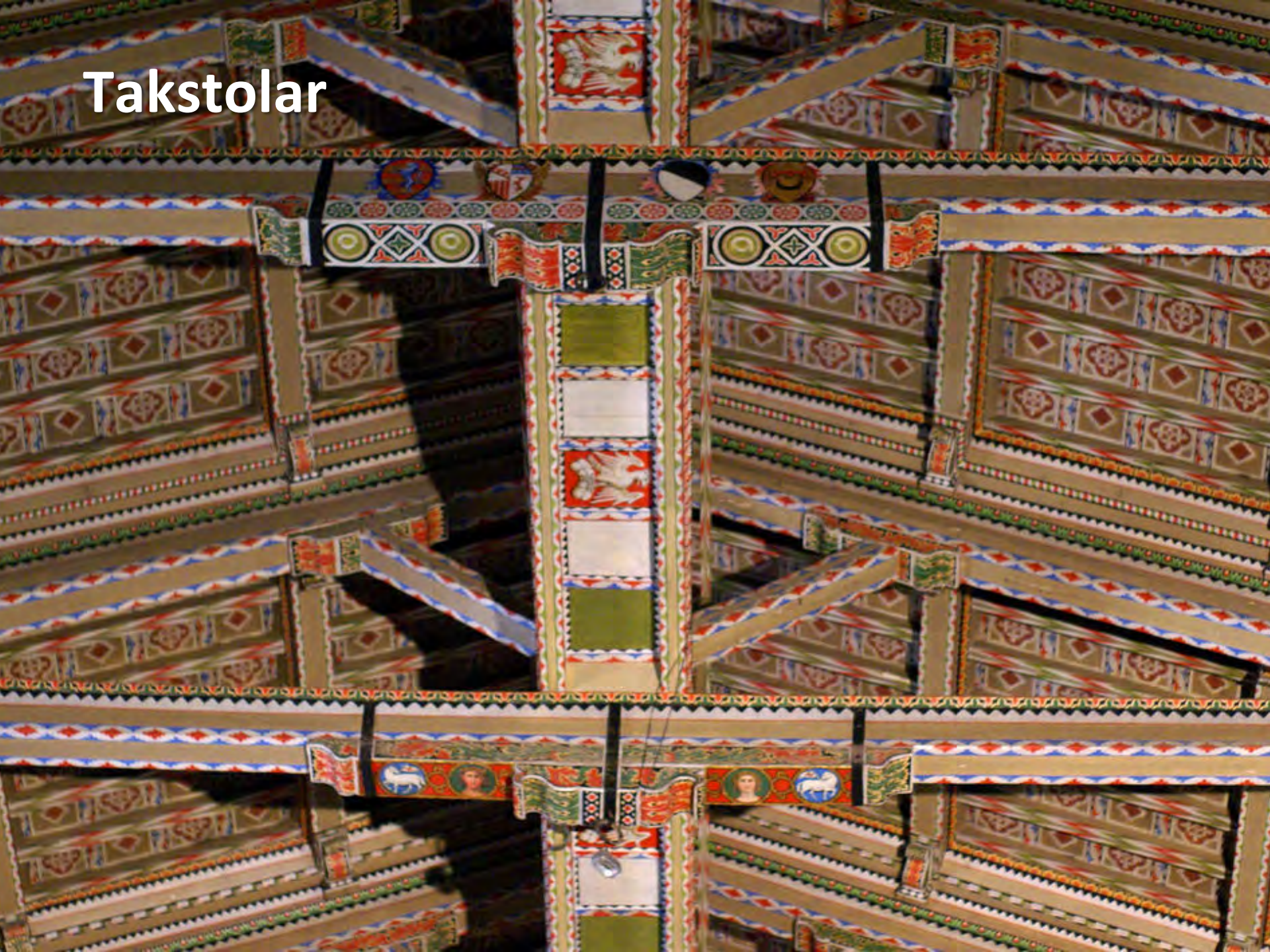


Skivan och pelaren

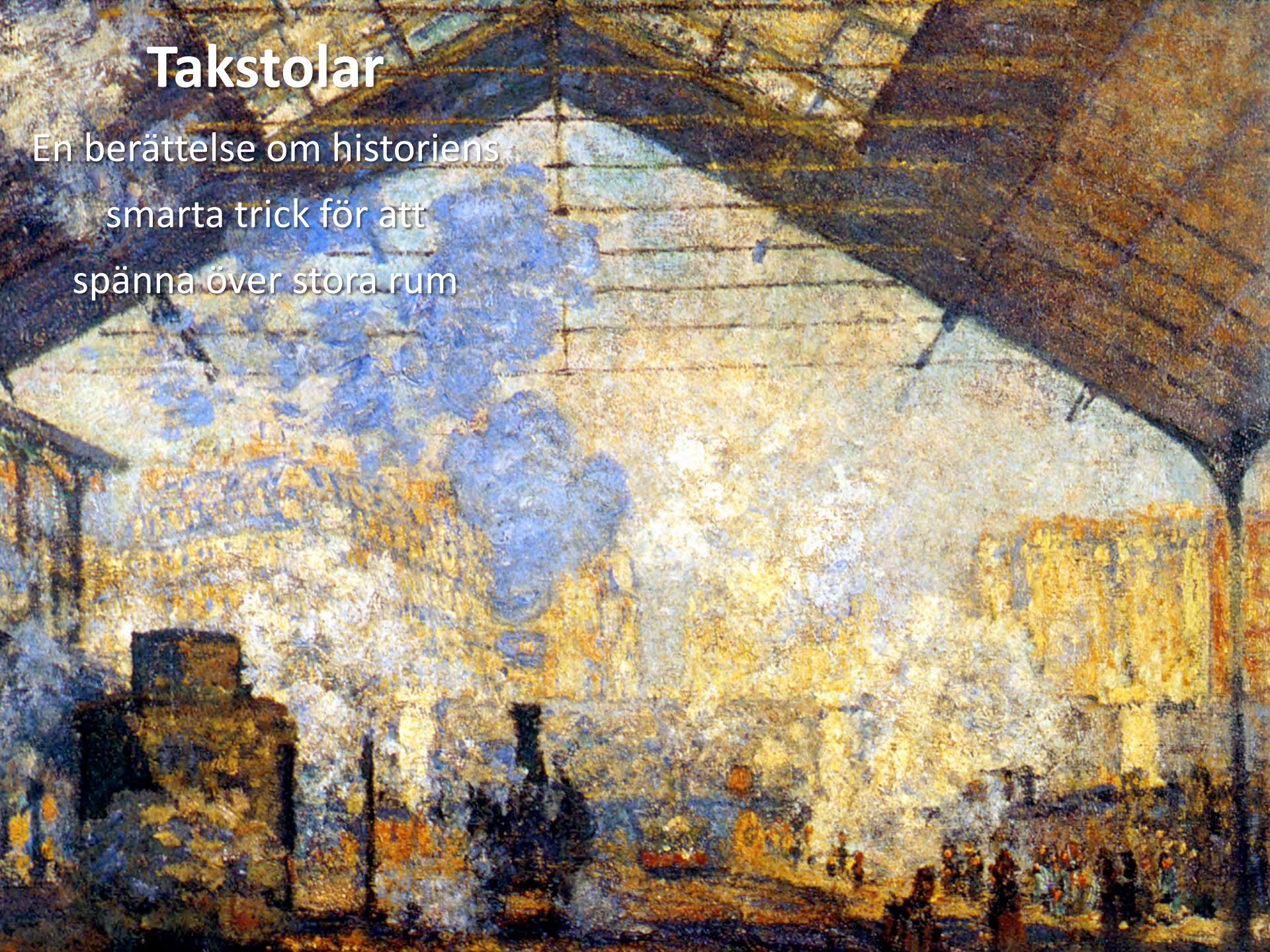


Takstolar

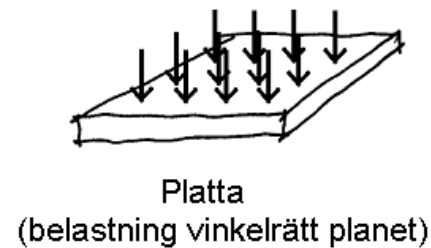
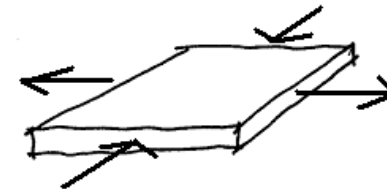
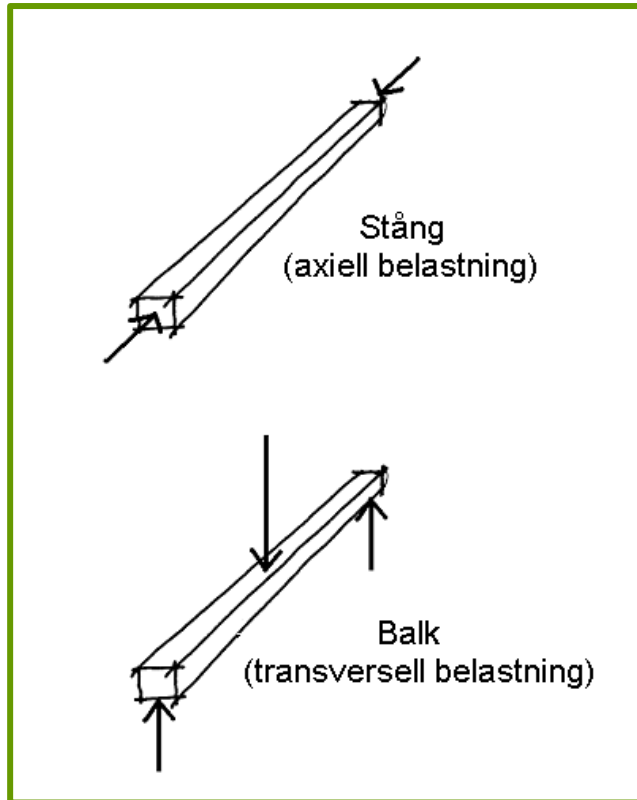


Takstolar

En berättelse om historiens
smarta trick för att
spänna över stora rum

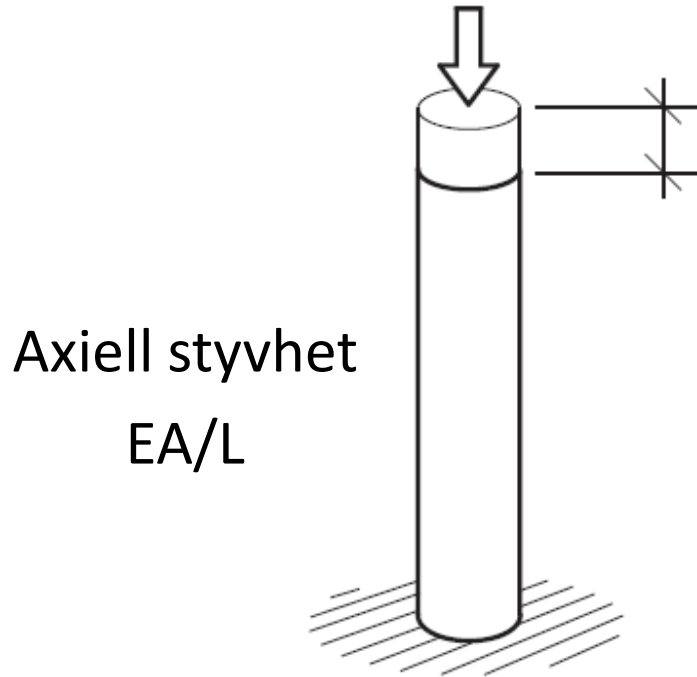


Konstruktionselementens verkningssätt (form + belastning)

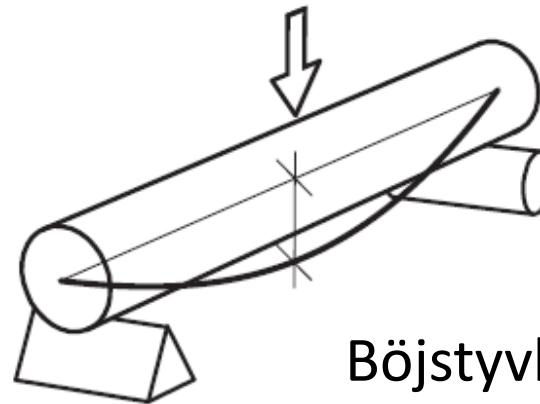


Takstolar

En berättelse om konstruktioners
effektivitet och om att reducera böjning
och bära med mer tryck och drag

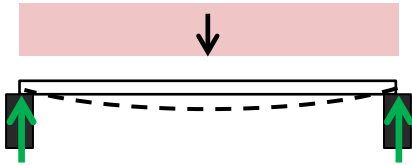


Axiell styvhet
 EA/L

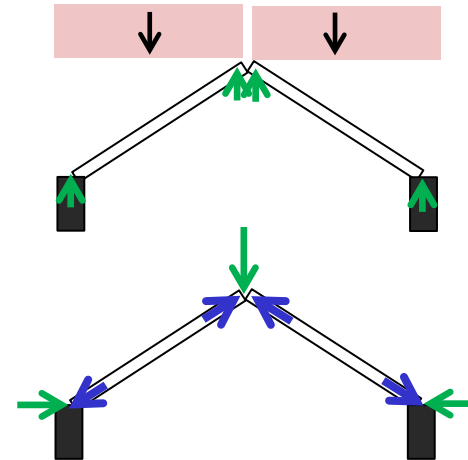


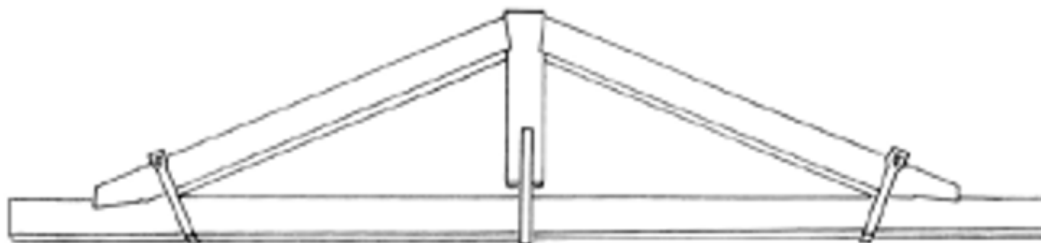
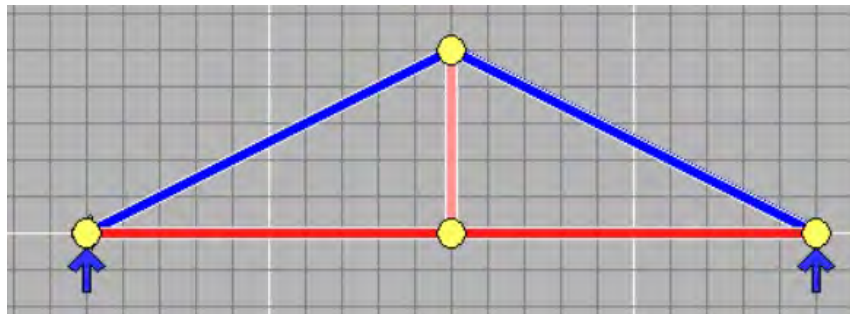
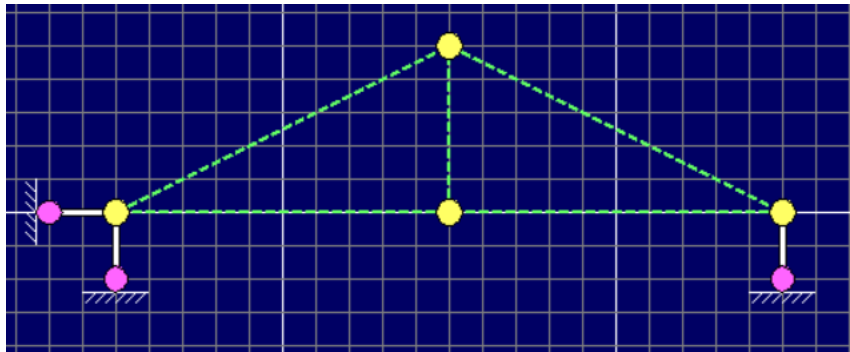
Böjstyvhet
 EI/L^3

Balkverkans ineffektivitet begränsar balkens längd

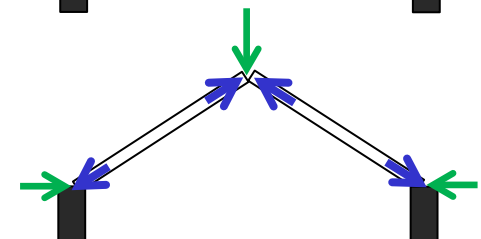
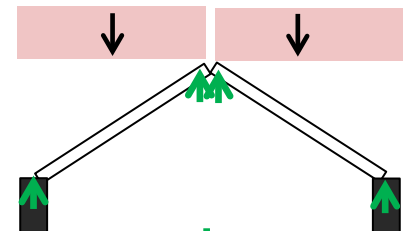
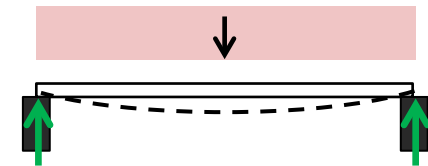


Hur förlänga en balk?



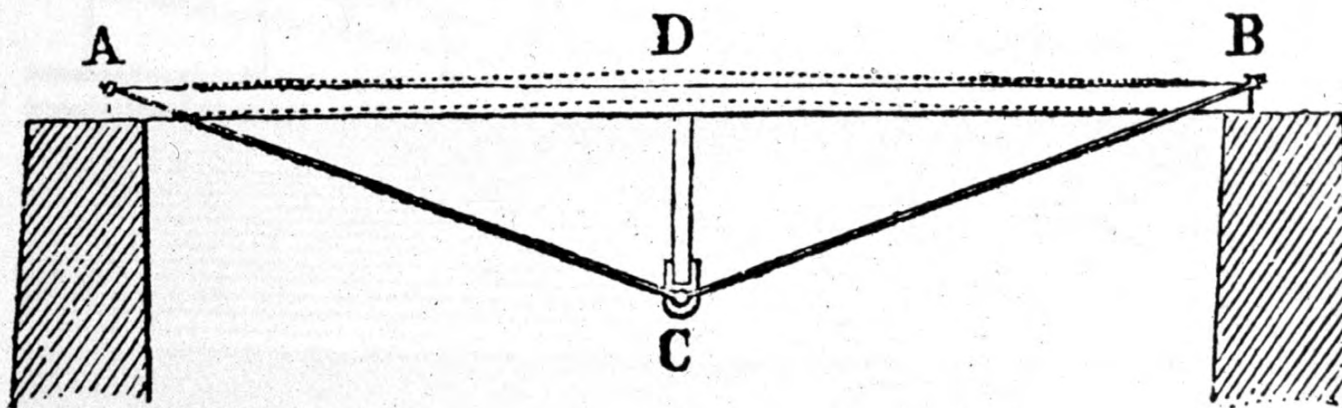


Hur förlänga en balk?

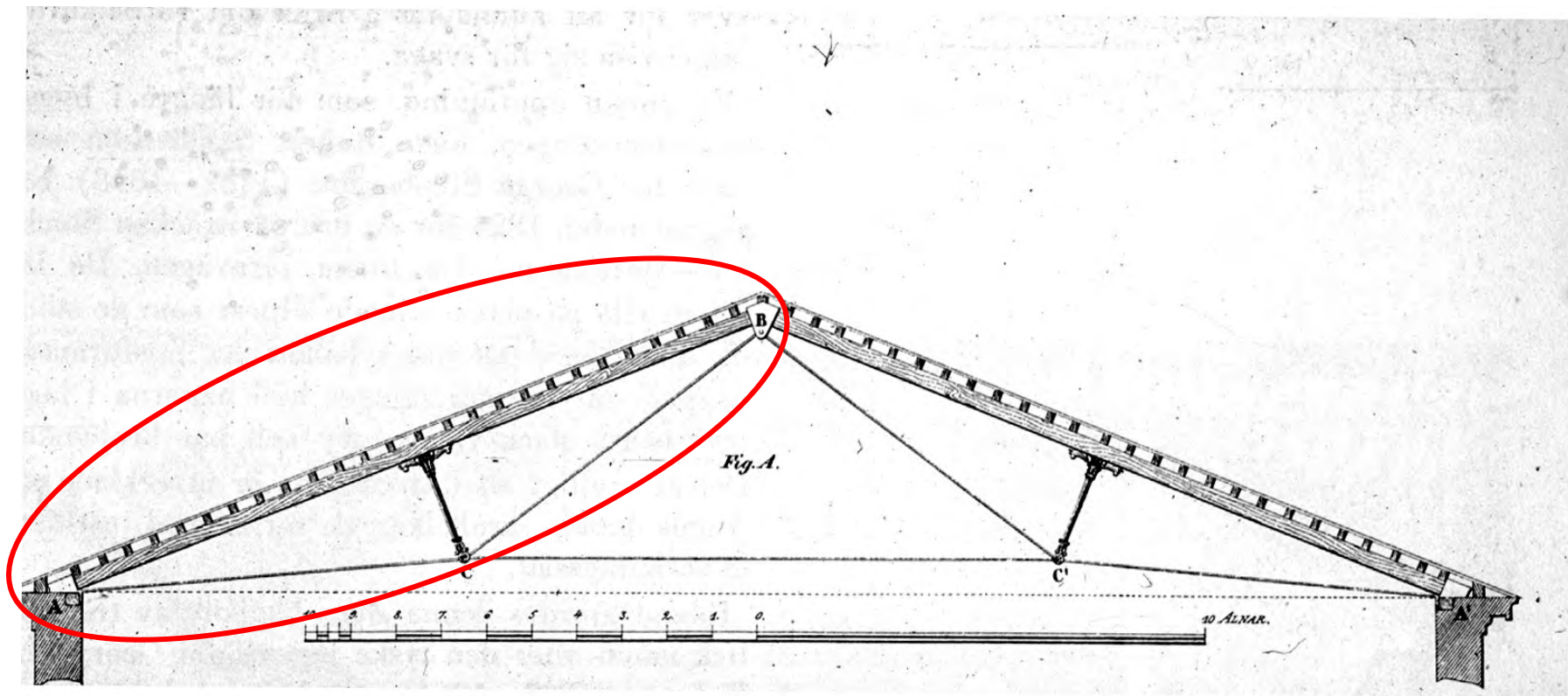




Hur förlänga en balk?



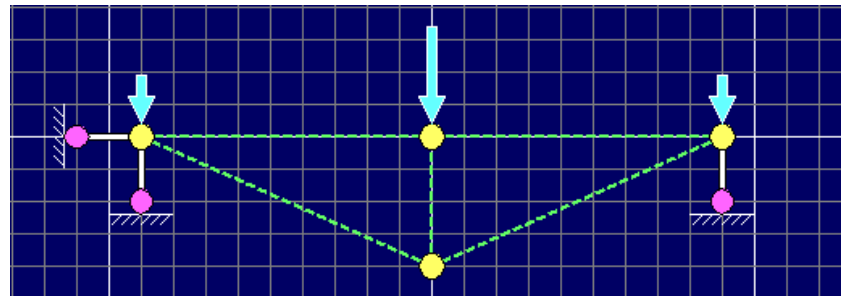
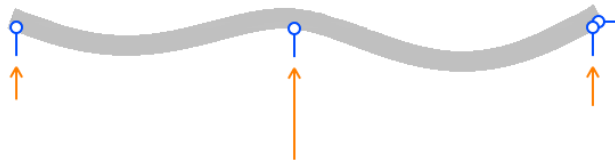
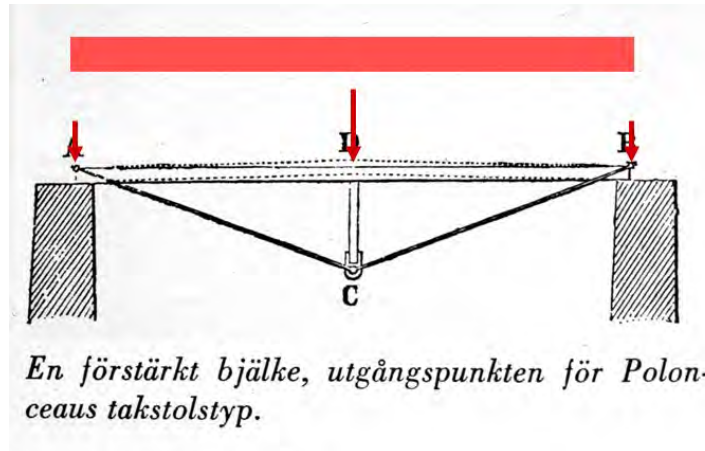
*En förstärkt bjälke, utgångspunkten för Polon-
ceaus takstolstyp.*

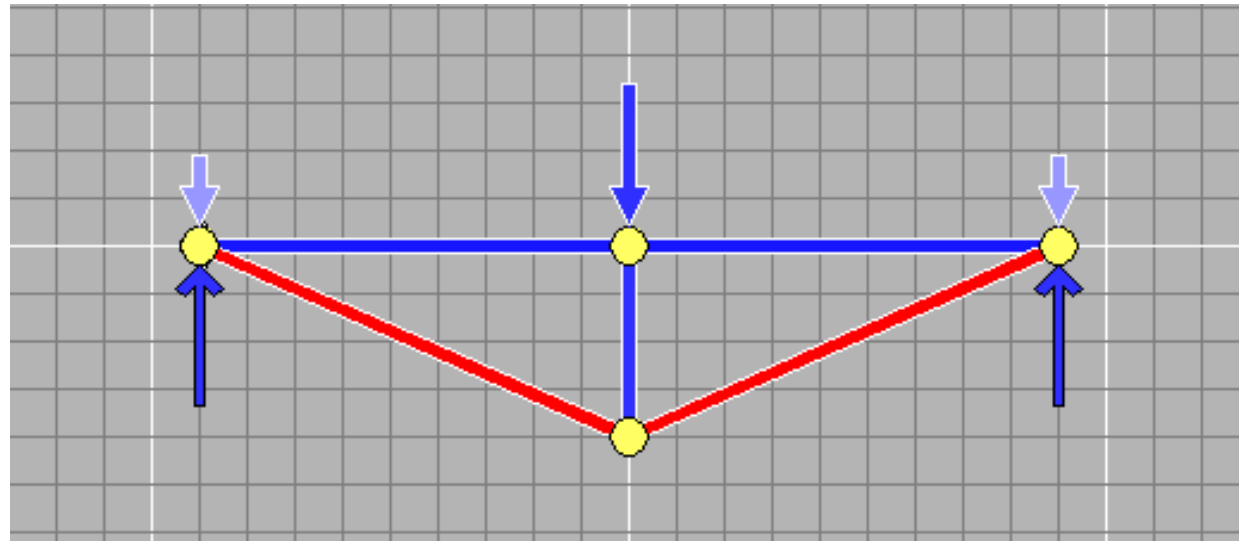
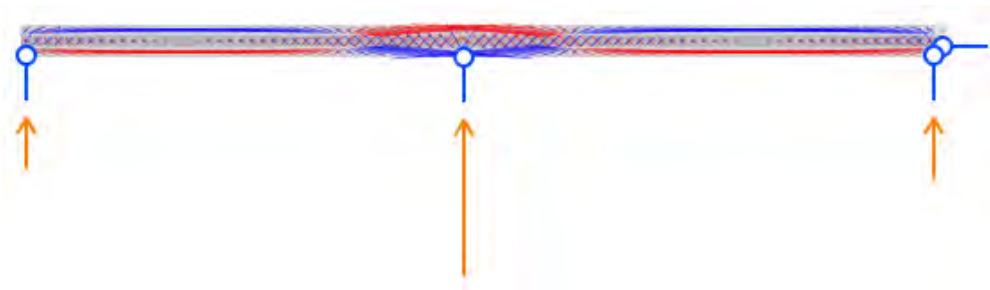


Ur Elias Cornell Byggnadstekniken 1970

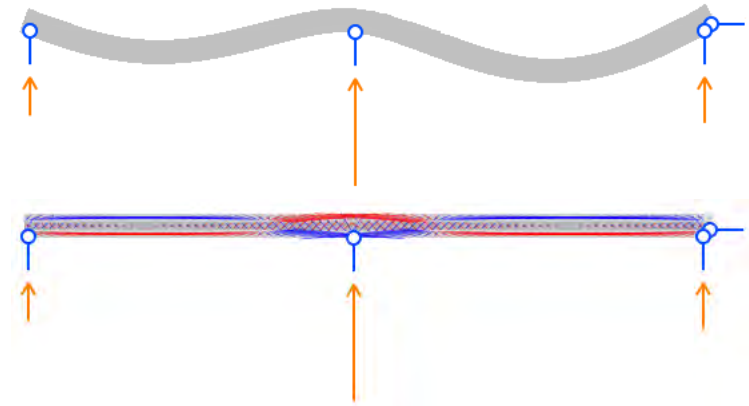
Idé

att reducera böjning och bära mer med tryck och drag

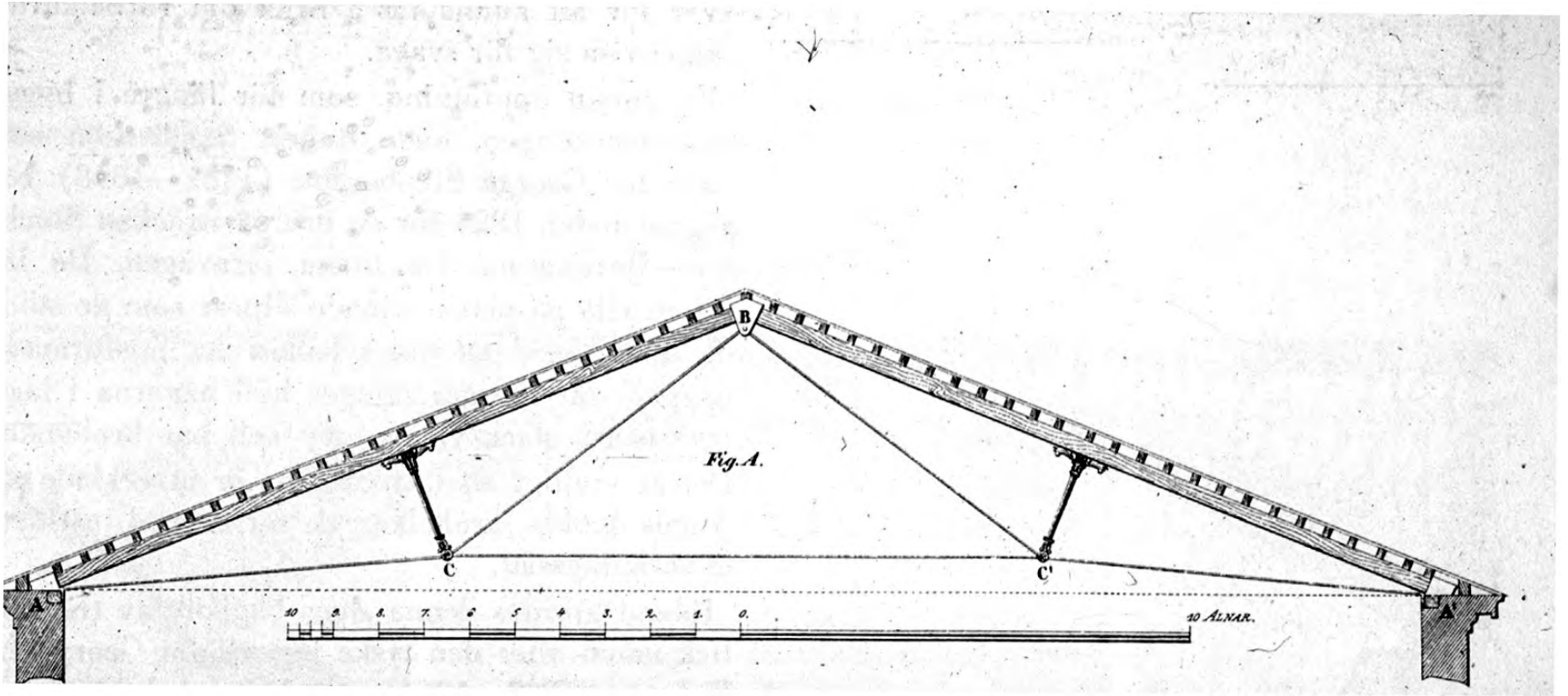




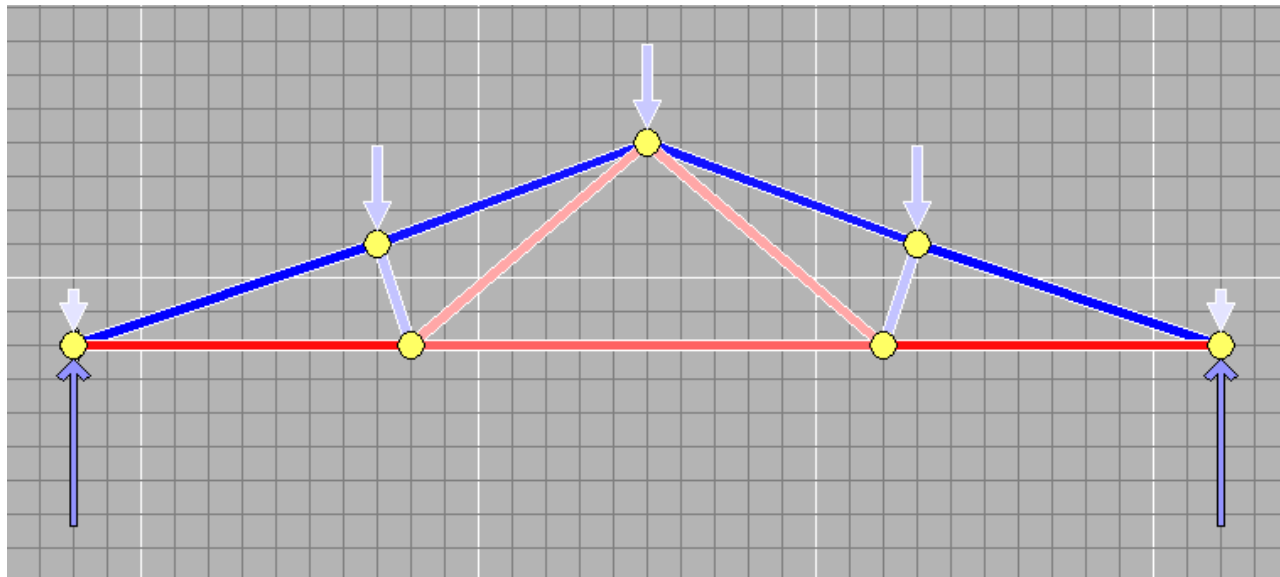
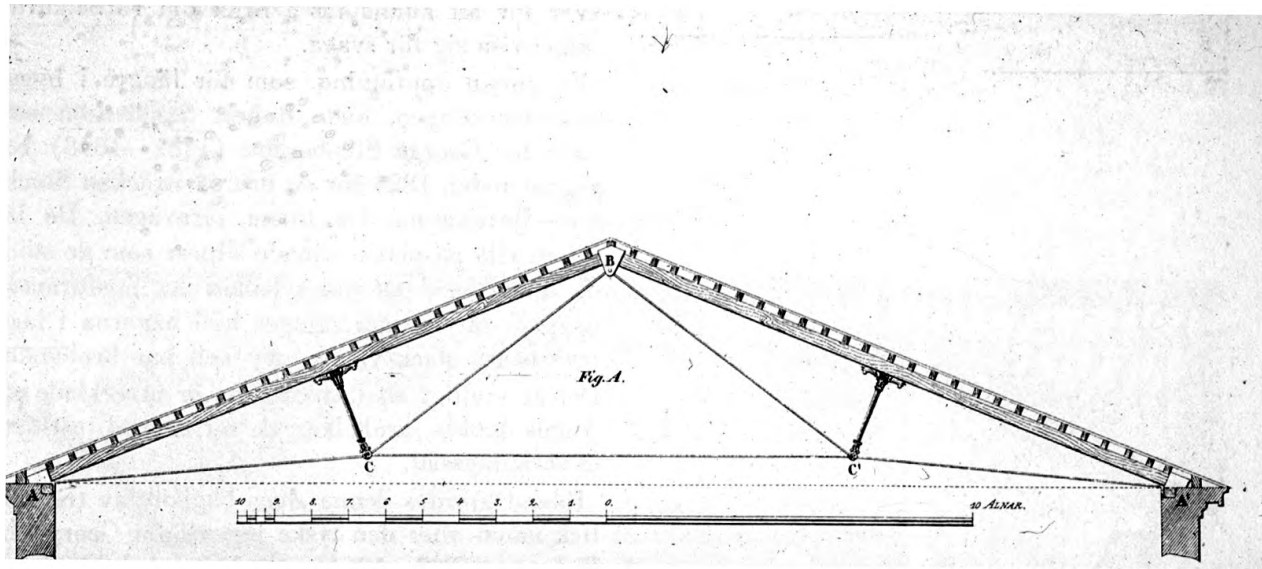
**Skarv där krökningen
(momentet) är noll**



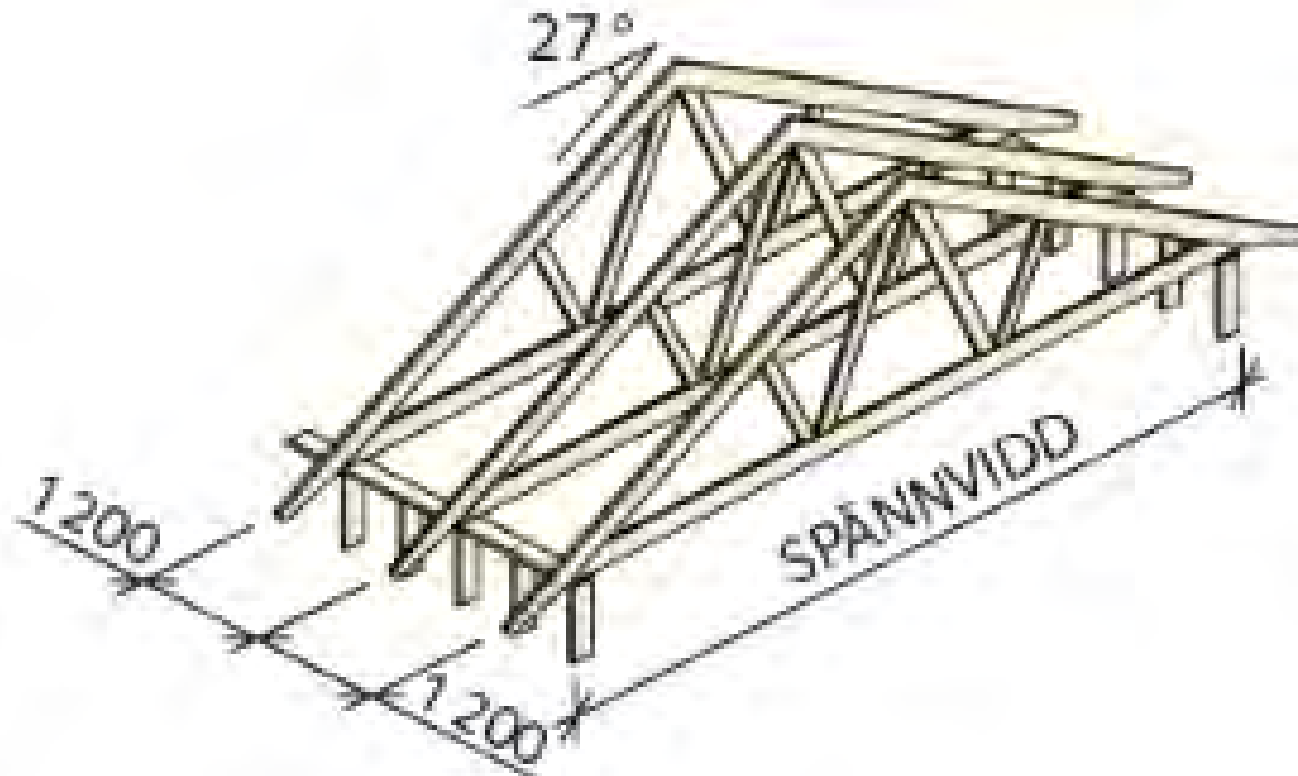
Polonceautakstol



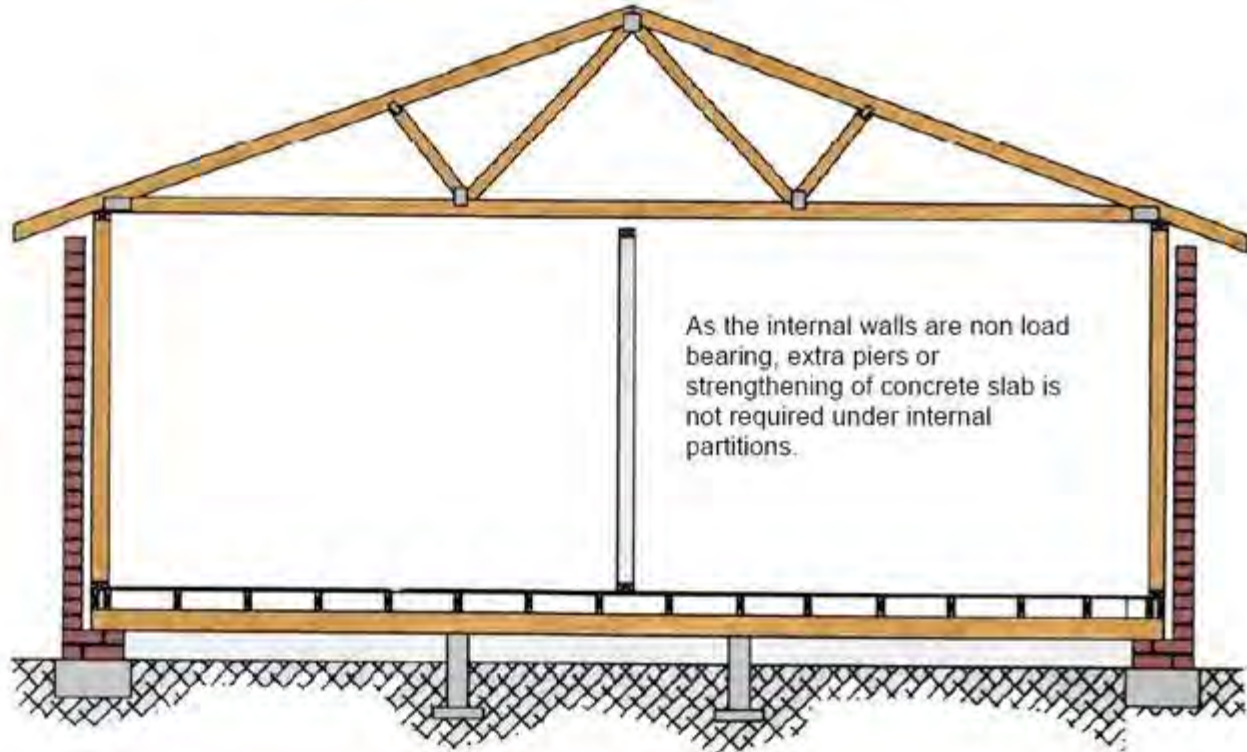
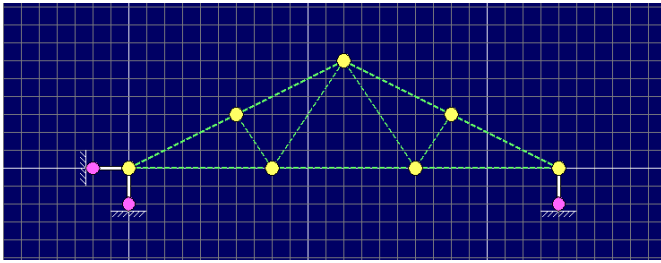
År 1839 uppfann **Camille Polonceau**
en metod för att spänna över stora rum
"one of the most successful roof designs
of the nineteenth century"



W - takstol

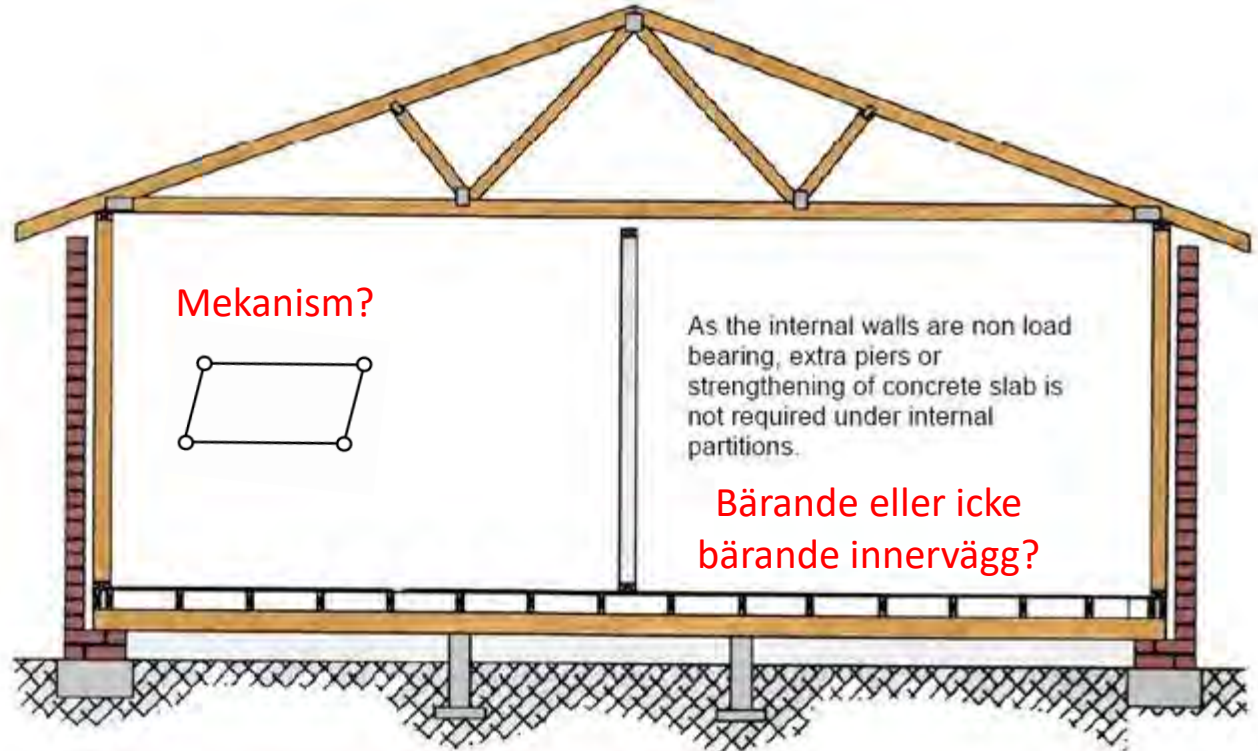
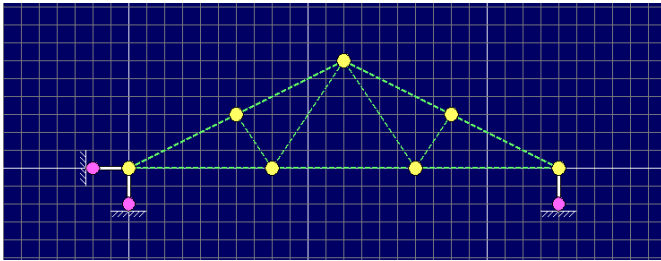


ur Träguiden



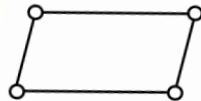
With trussed roof construction, only external walls are load bearing, making the design of floor structure simple and inexpensive.

As the internal walls are non load bearing, extra piers or strengthening of concrete slab is not required under internal partitions.



With trussed roof construction, only external walls are load bearing, making the design of floor structure simple and inexpensive.

Mekanism?



As the internal walls are non load bearing, extra piers or strengthening of concrete slab is not required under internal partitions.

Bärande eller icke bärande innervägg?

Plintarnas placering och varför extra plintar?

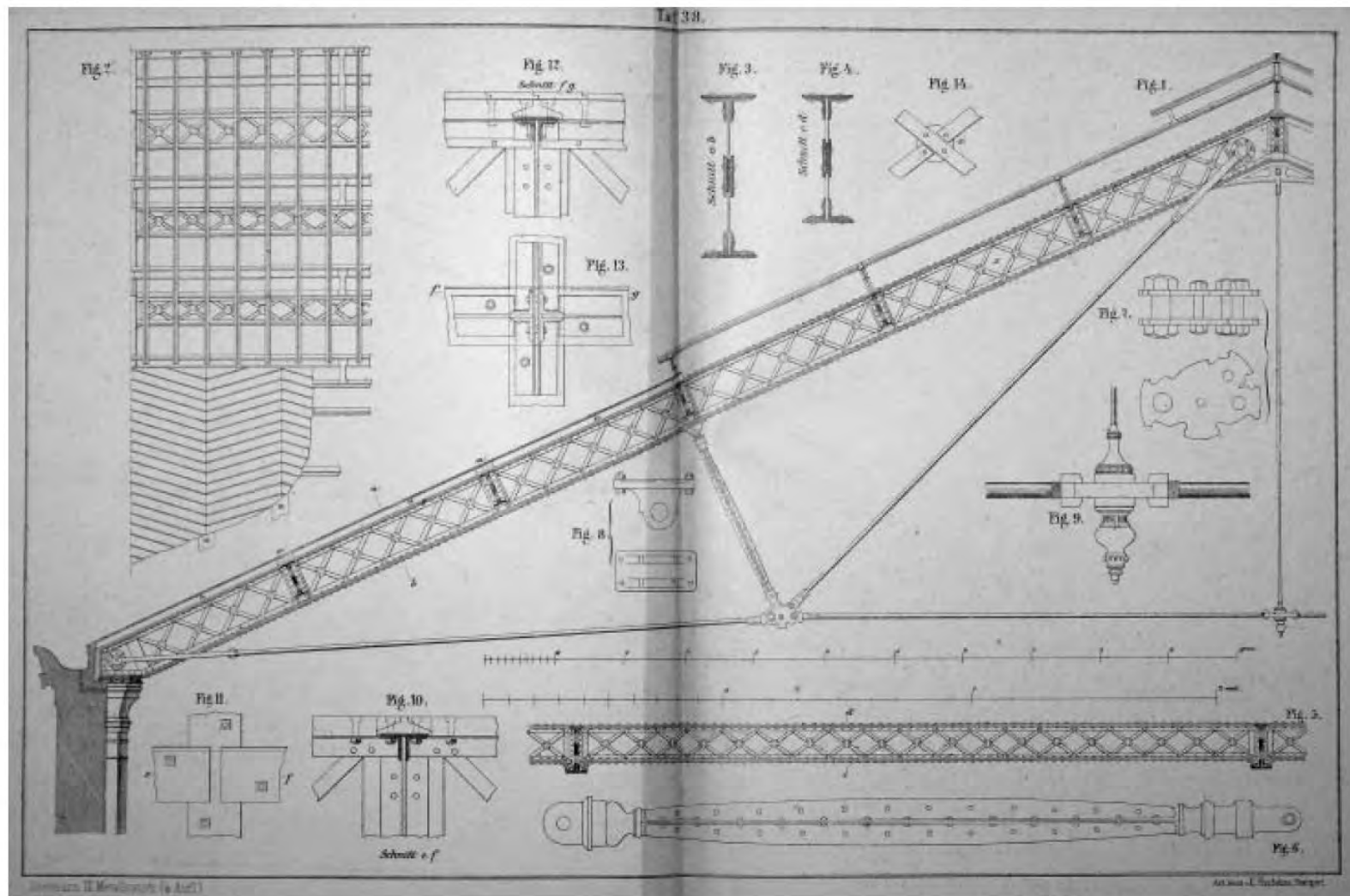


FIGURE 15 Lyon-Perrache, roof of the railway station (completed in 1855). From Breymann 1877, plate 38.

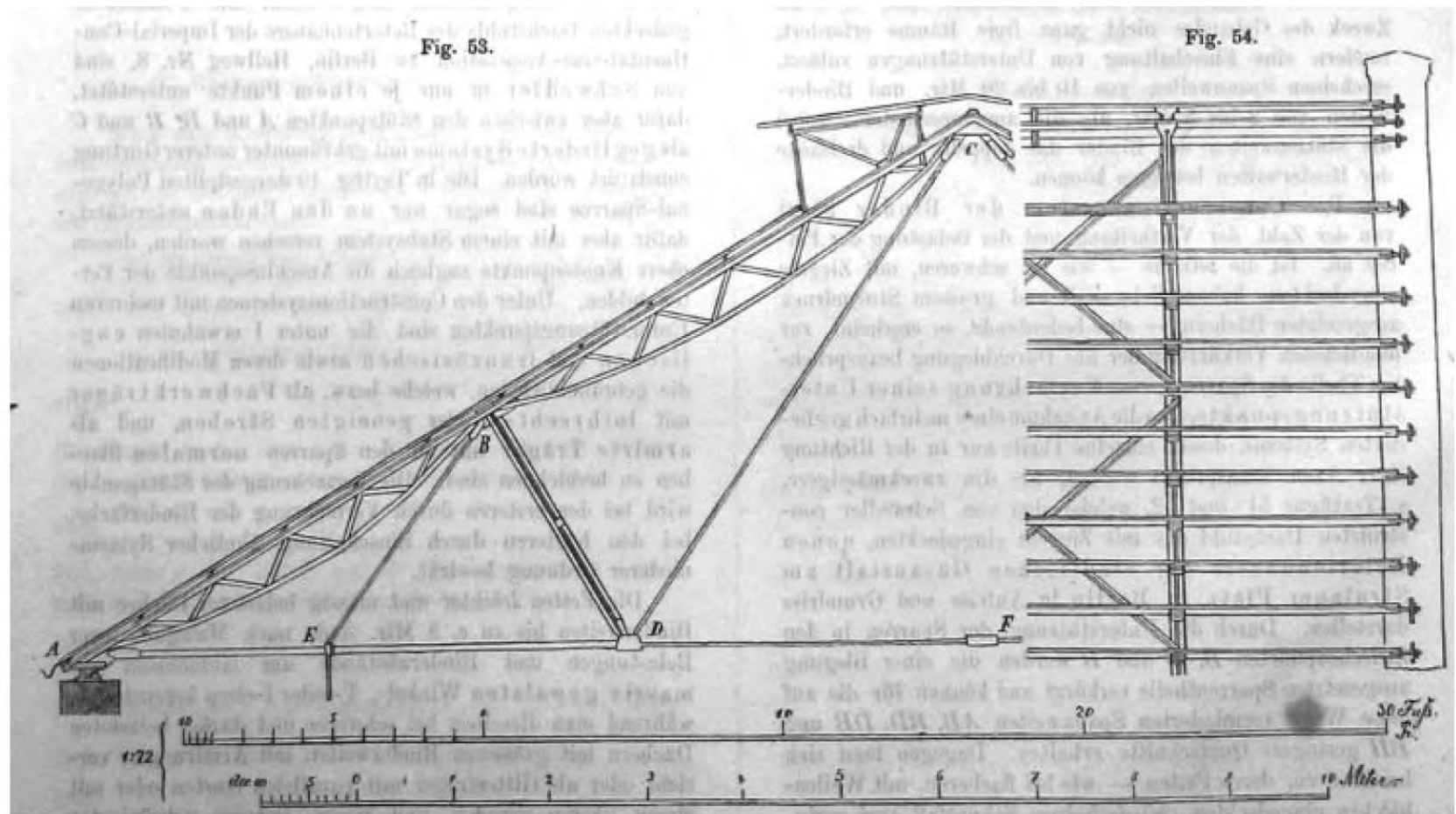


FIGURE 17 Berlin, roof of a workshop of the Imperial Gas Association (designed by J. W. Schwedler). Transverse section (left) and ground plan (right). From Heinzerling 1876, p. 20.

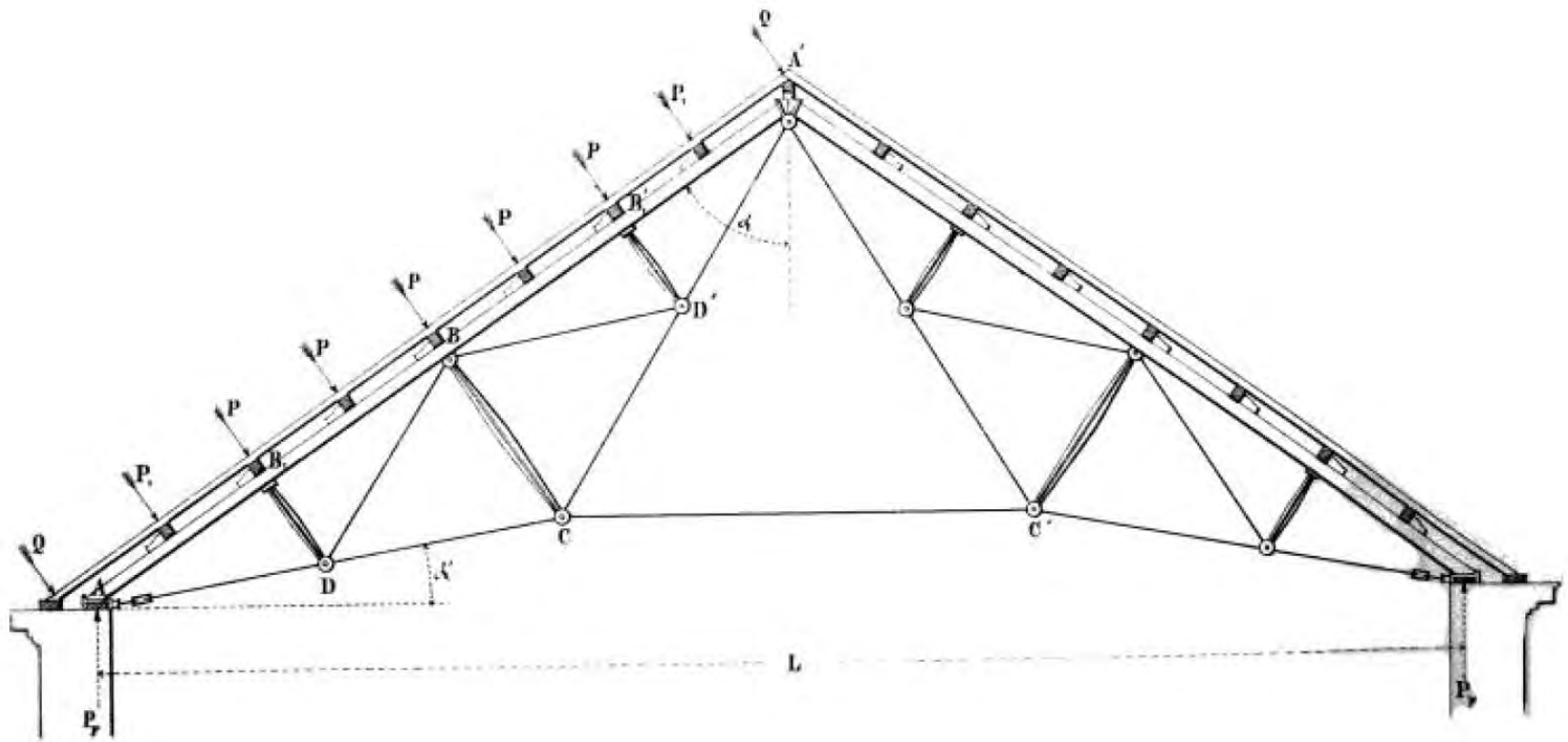
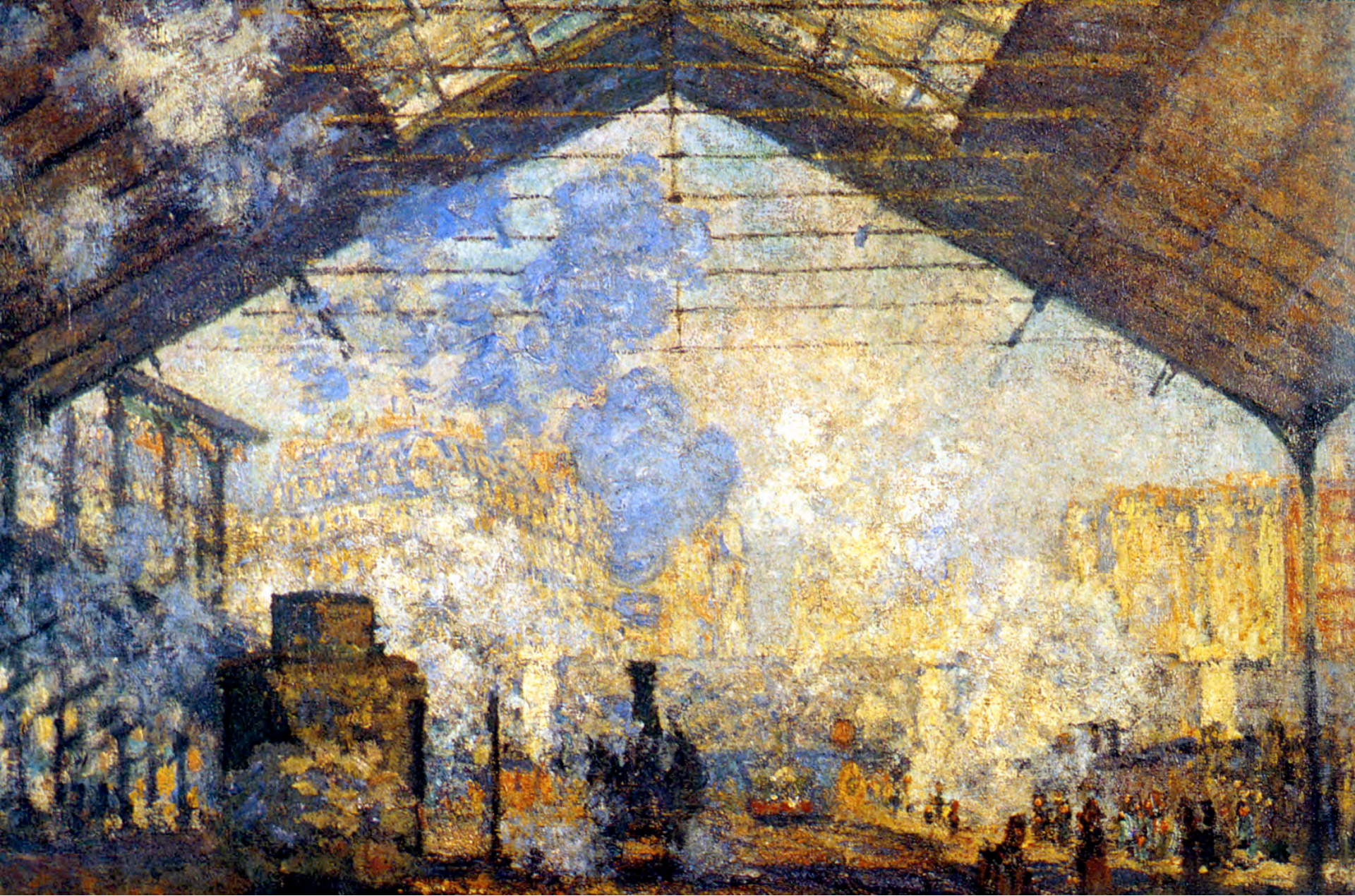
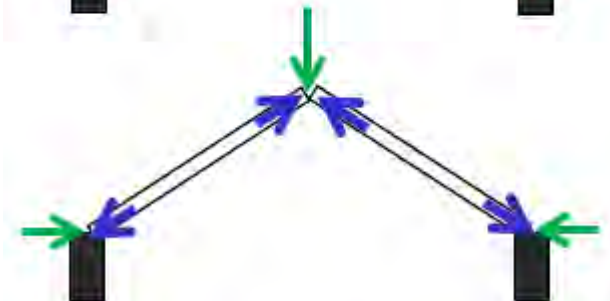
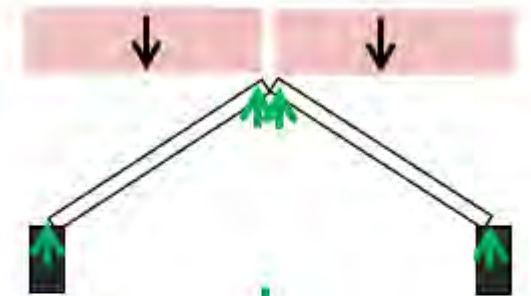
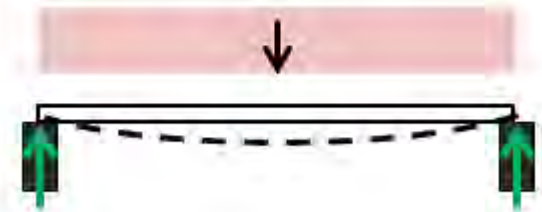
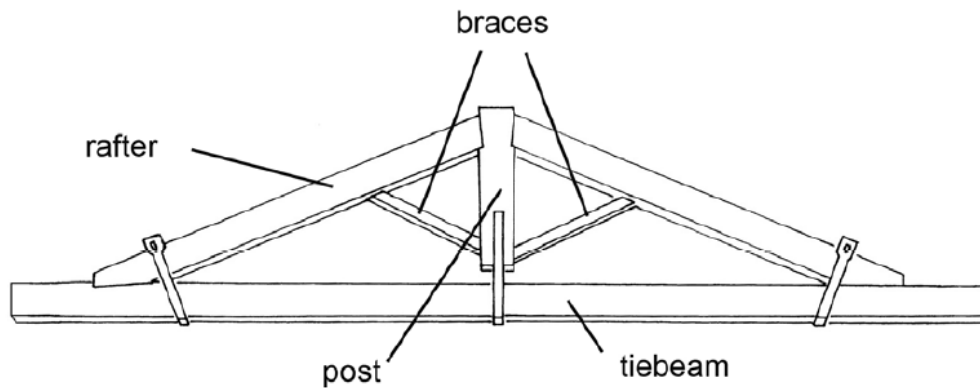
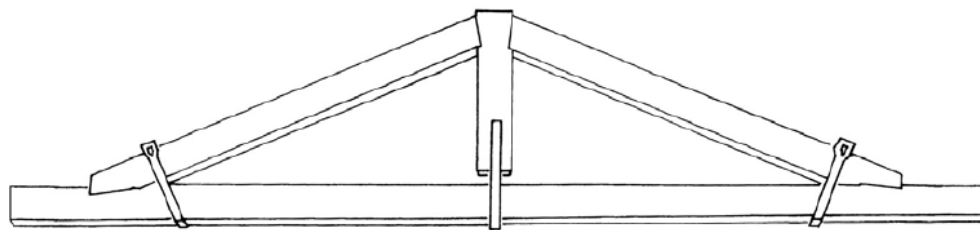


FIGURE 20 Polonceau roof with triply braced principal rafters. From Michon 1848, p. 290.



Claude Monet Gare Saint-Lazare

Hur förlänga en balk?



Saint Catherine's Monastery
Mount Sinai





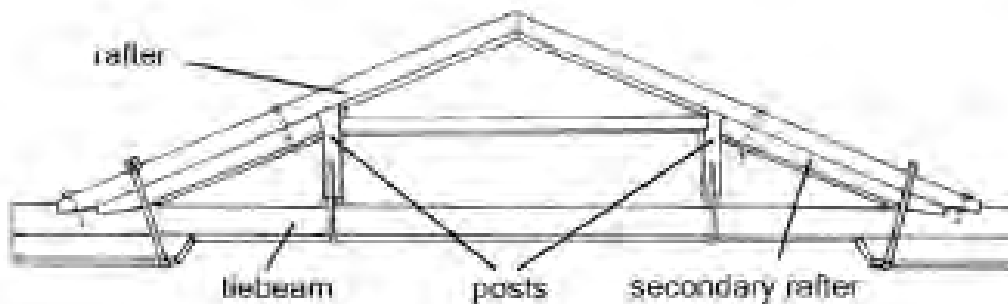
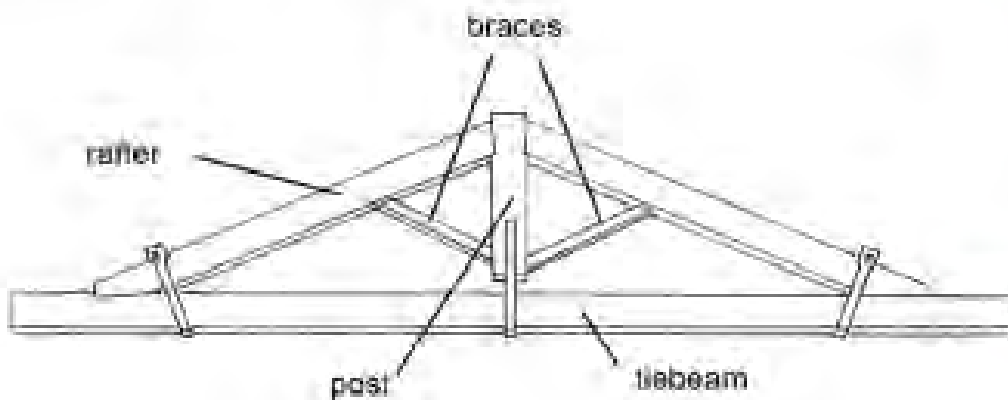
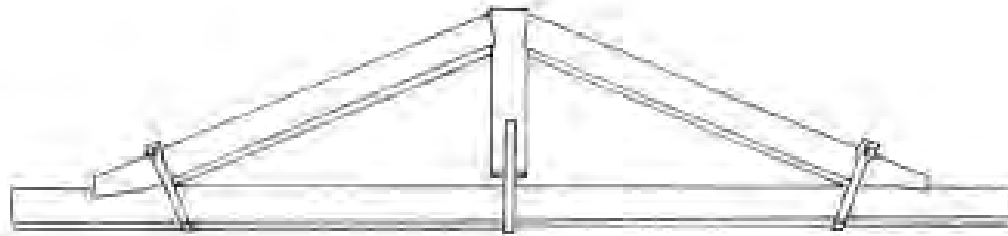
**Palazzo Vecchio
Florence**



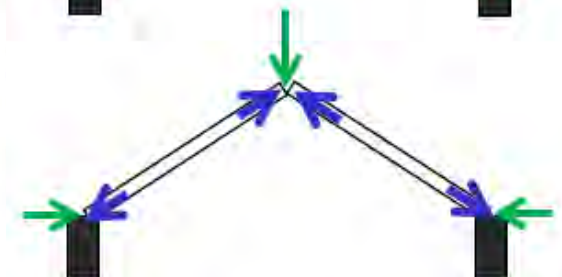
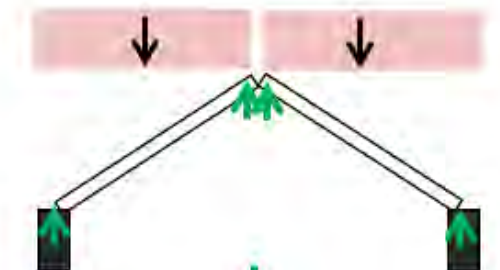
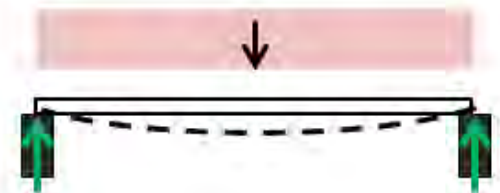




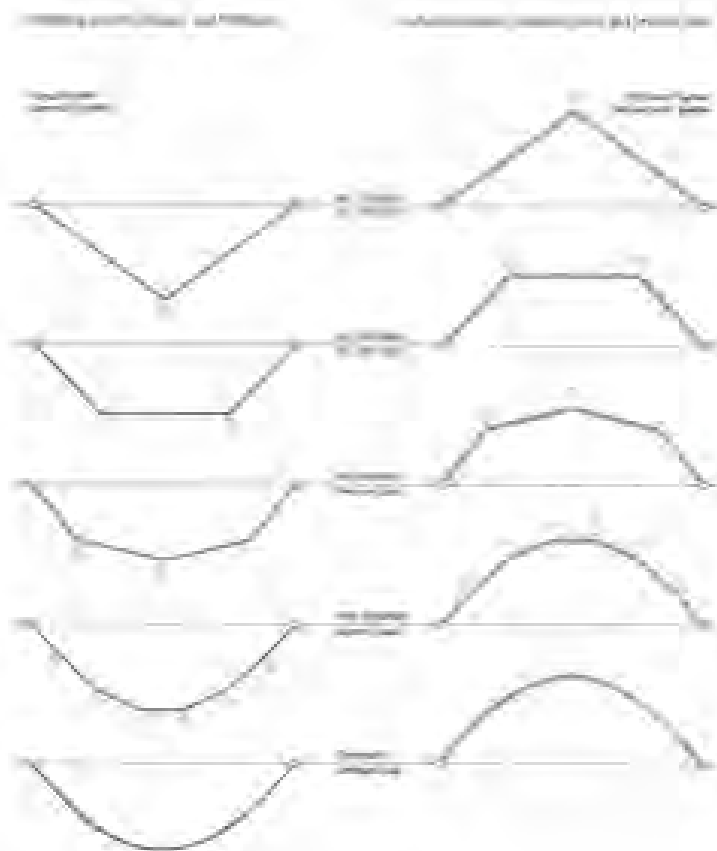




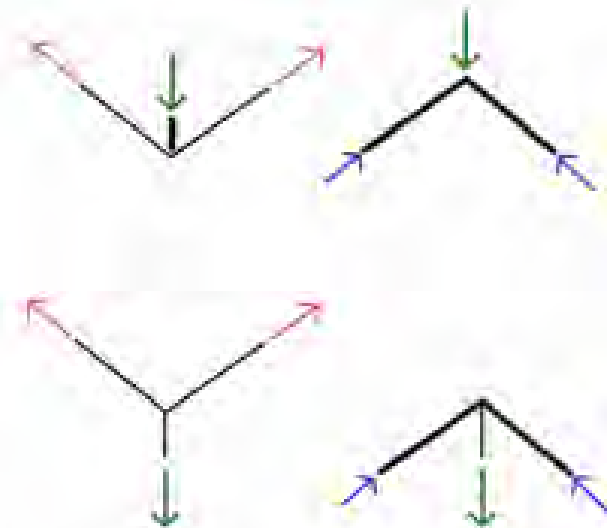
Hur förlänga en balk?



Linan och bågen (kedjekurvan och trycklinjen)

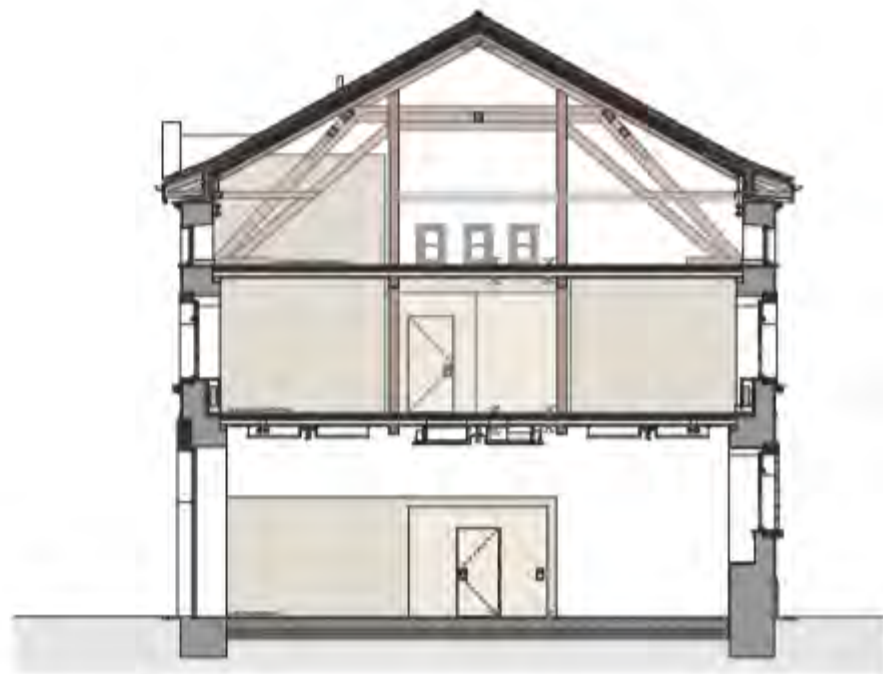
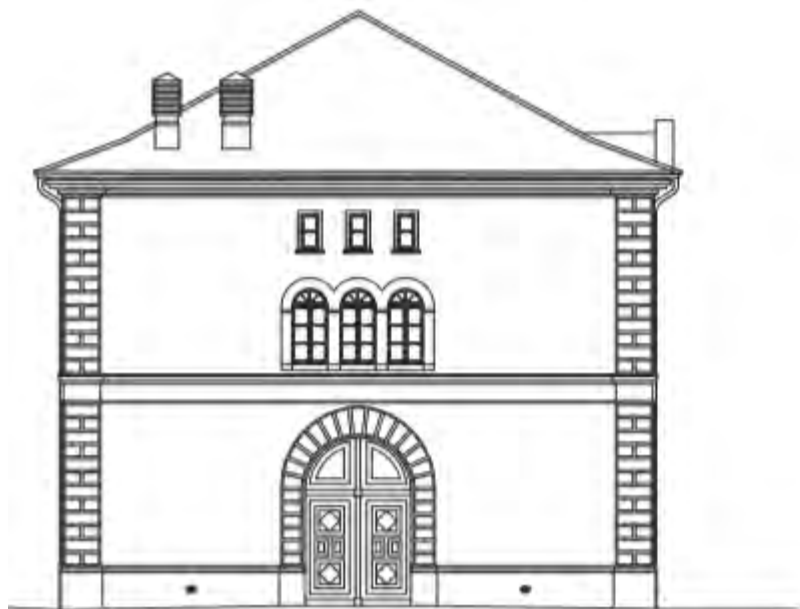


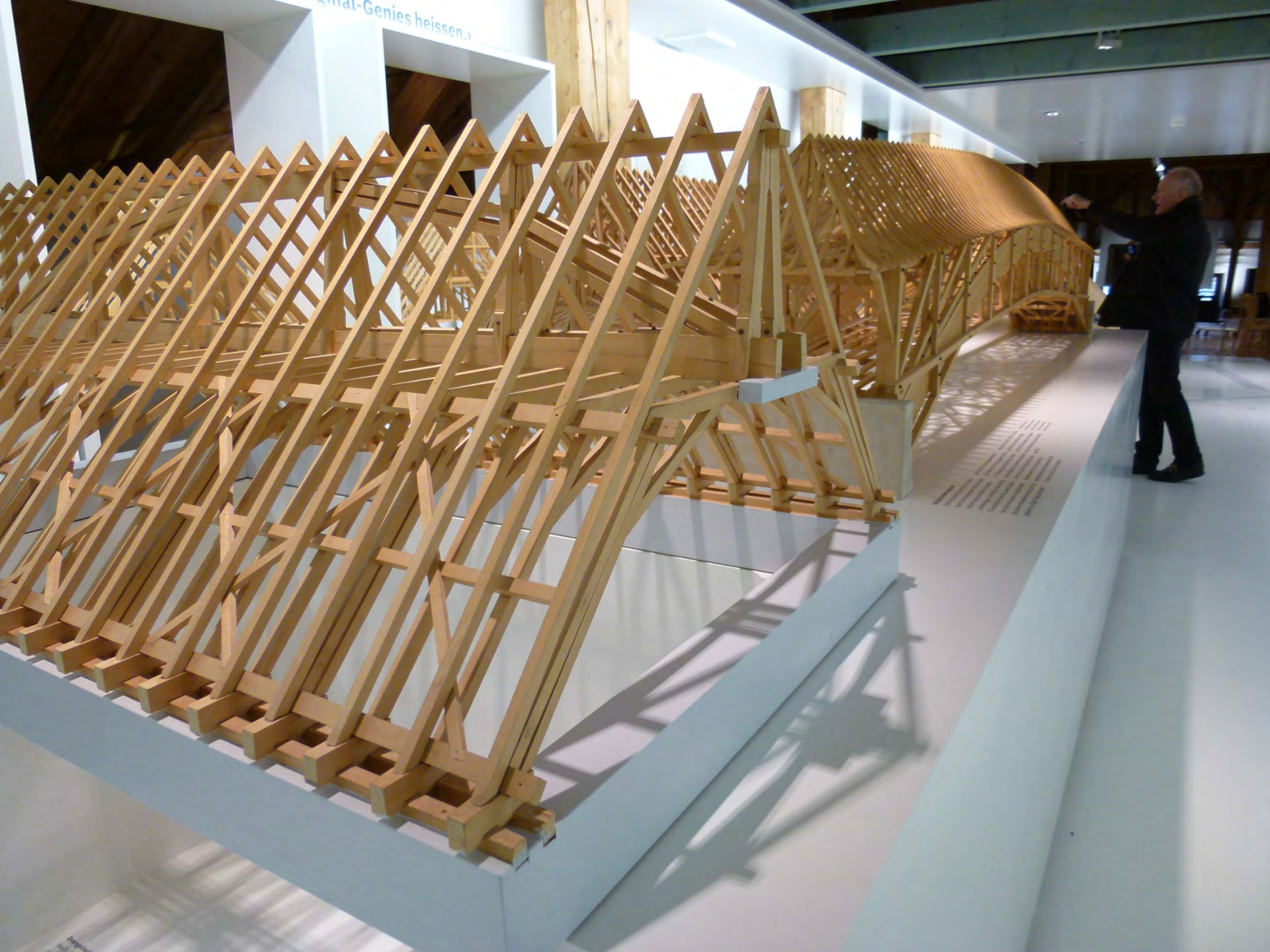
Grundläggande mekanism



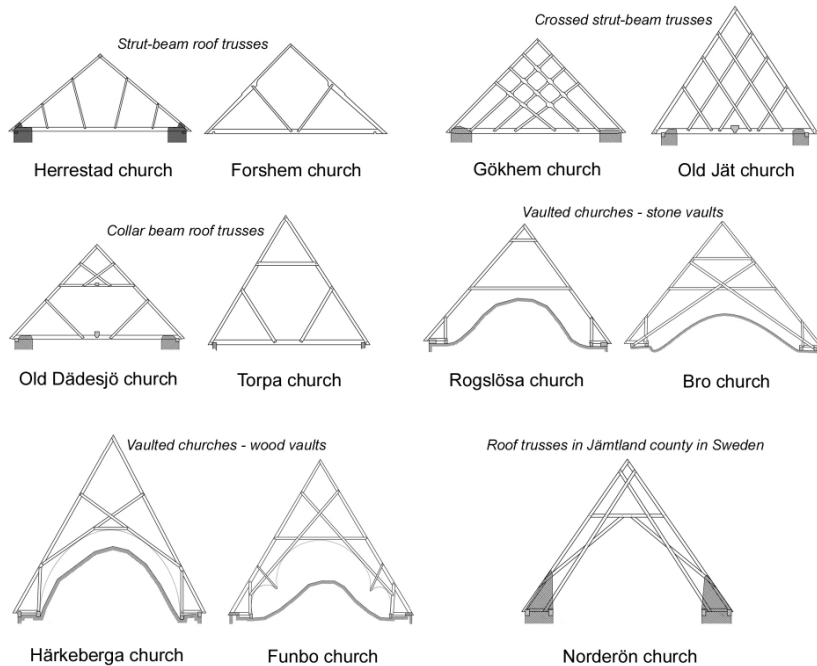
Partikeljämvikt

Strukturen bär den yttre kraften
genom rent drag eller rent tryck





Mechanical patterns and structural form





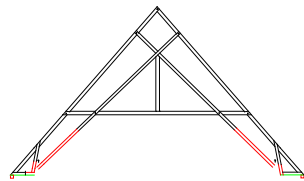
**Hidden behind ceilings
Sweden have an extensive heritage
of old timber structures**

**There are today 120 known
still standing timber roof structures
dated as medieval**

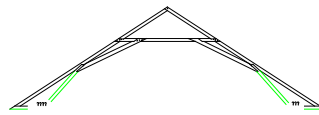
17 of them from before 1150 a.D.



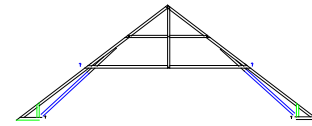
Old Jät Church



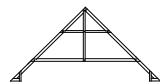
ALMUNDSRYDS KYRKA, 1828 - 31



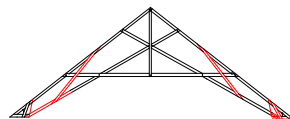
BERGA KYRKA, 1819 - 21



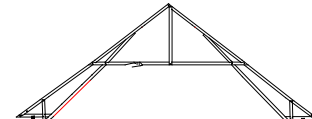
HJORTSBERGA KYRKA, 1838 - 40



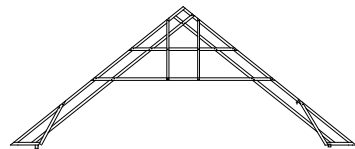
HÅRLUNDA KYRKA, 1822 (ombyggt)



RYSSBY KYRKA, 1844

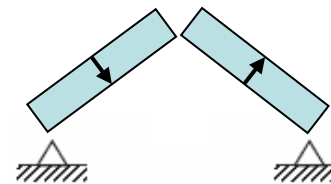
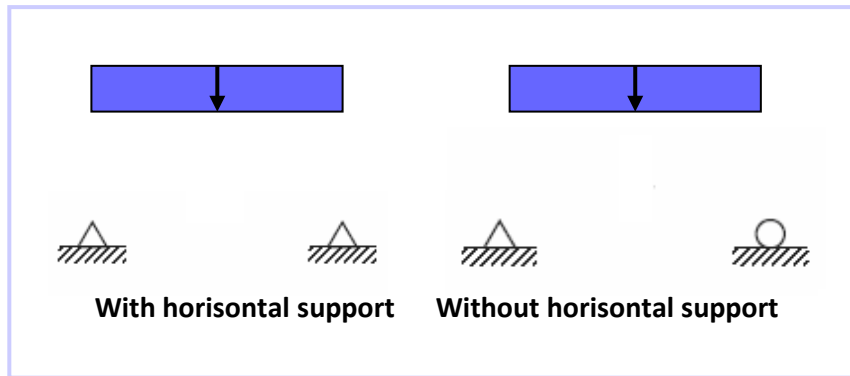


TINGSÅS KYRKA, 1853

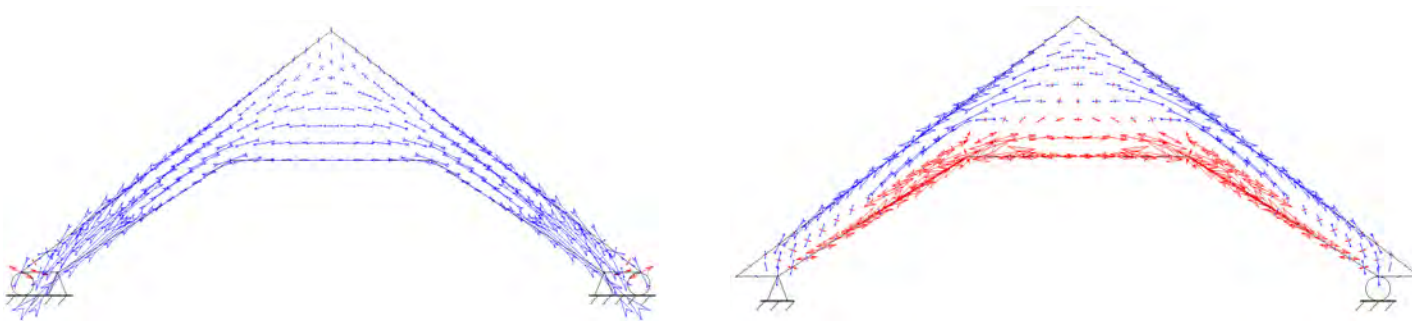


URSHULTS KYRKA, 1808 - 10

In this environment **Ylva Sandin**
analyses how the inner form
respond to a redistribution of internal forces
due to lack of horizontal support

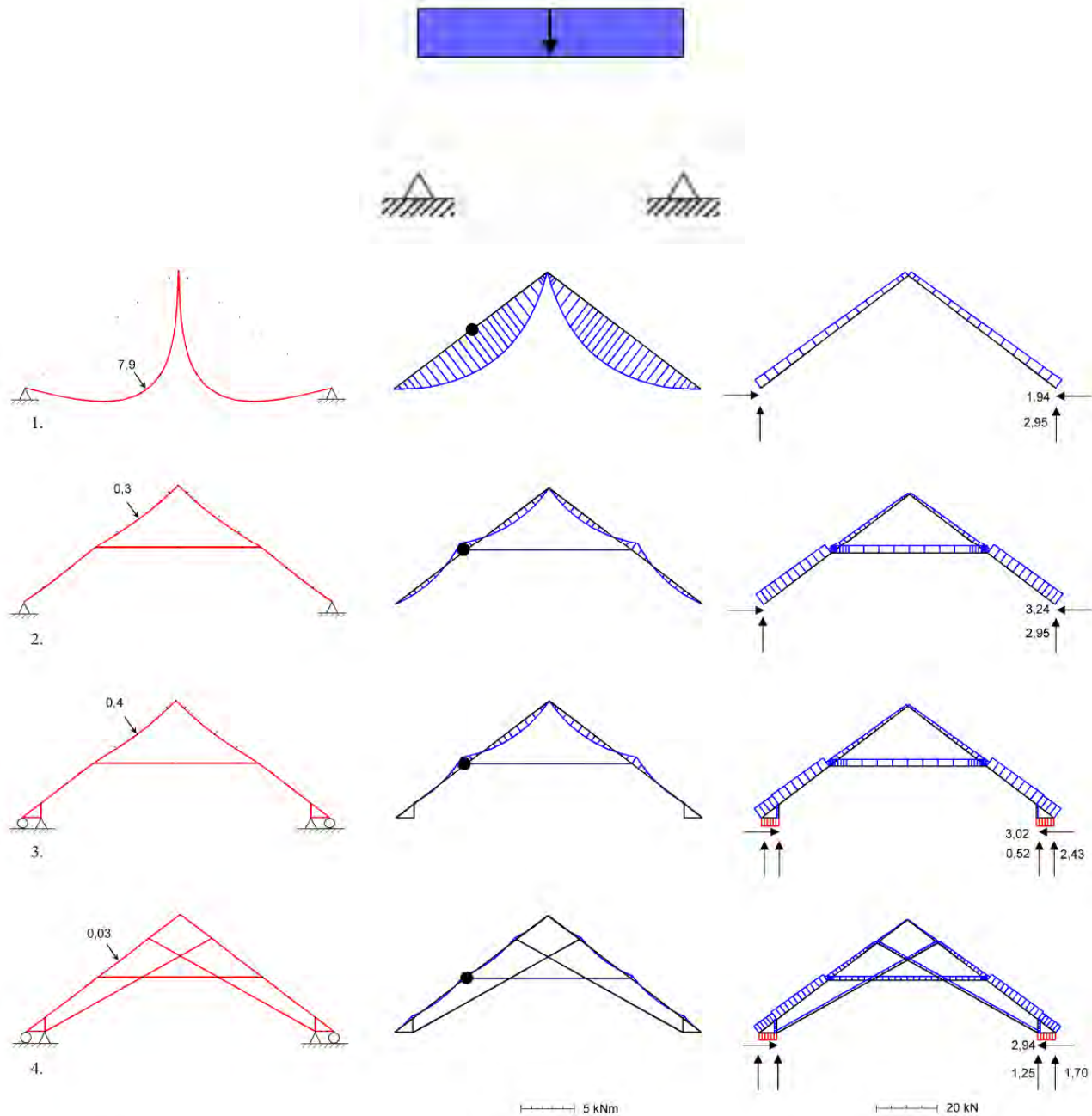


Structural tasks



**Patterns of
load paths**

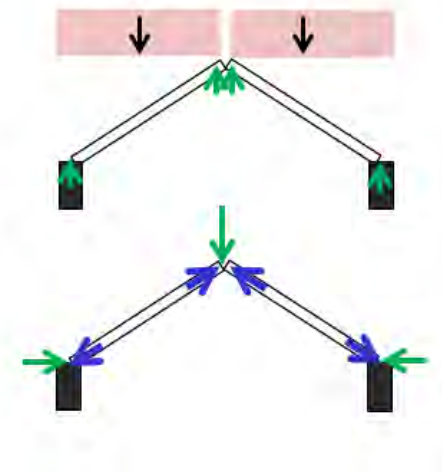
A successive redistribution of stress pattern



A. Deformationsfigur

B. Böjmoment

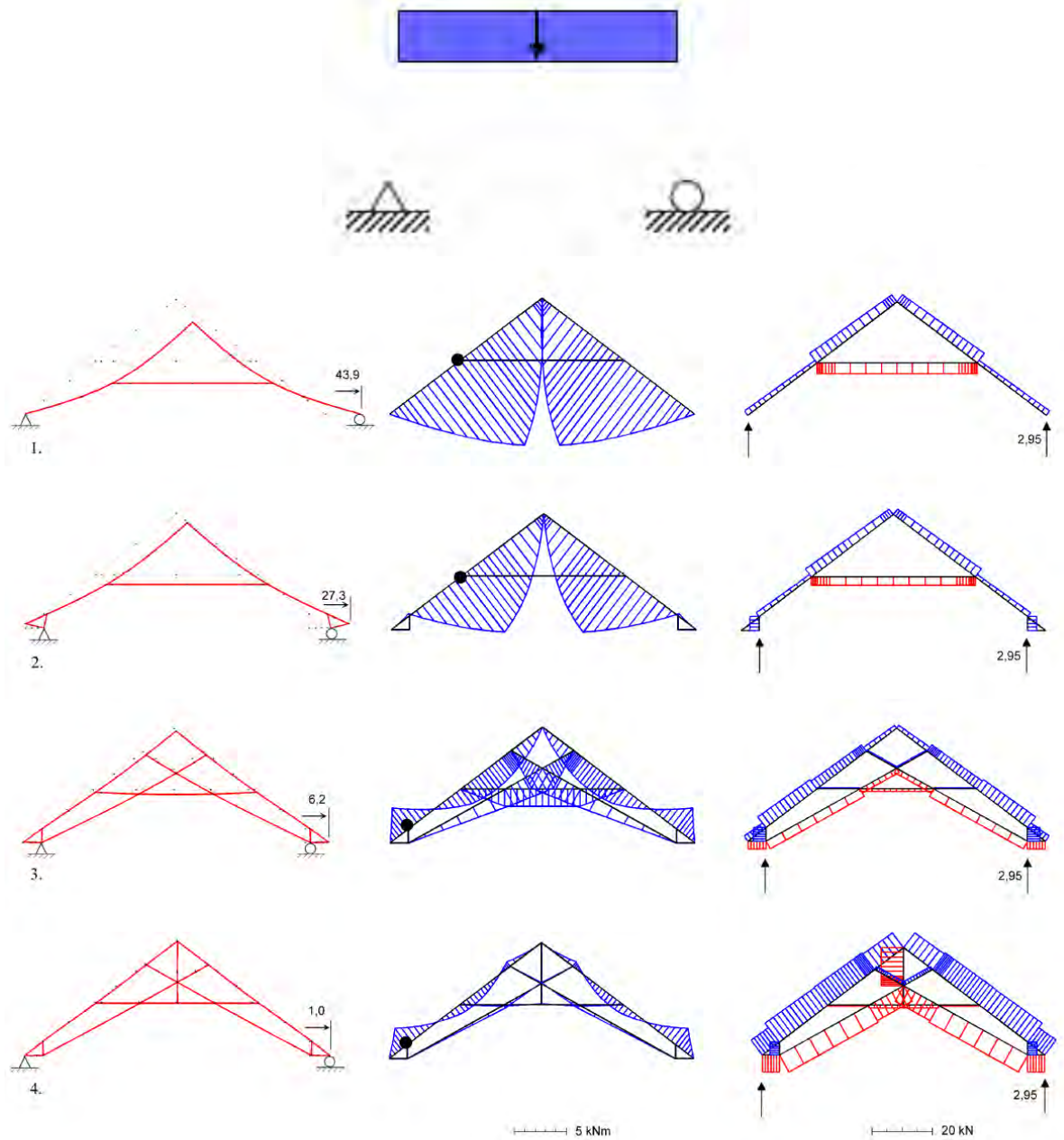
C. Normalkrafter och stödkrafter



The presence
of an inner structure
enables
a redistribution

from less efficient
bending action

to more efficient
axial action



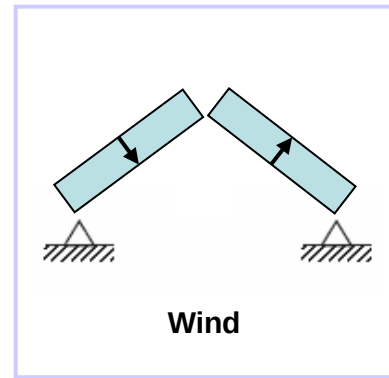
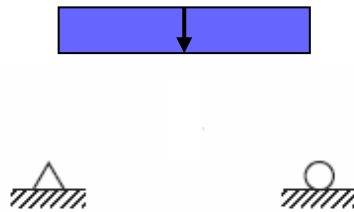
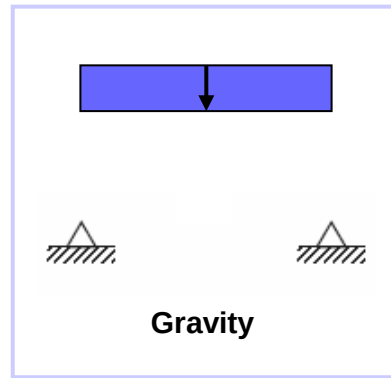
A. Deformationsfigur

B. Böjmoment

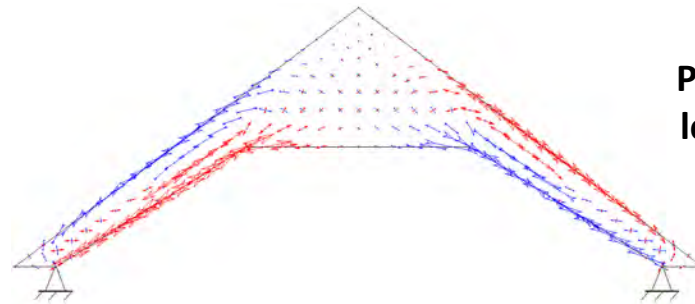
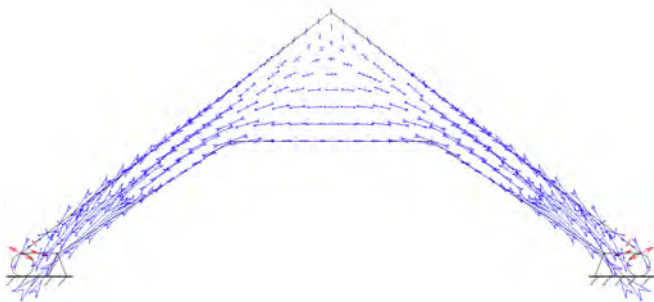
C. Normalkrafter och stödkrafter

Ylva Sandin

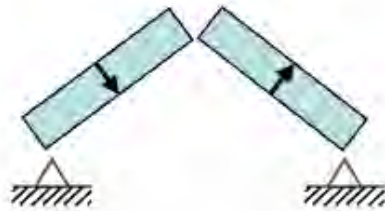
analyses the efficiency of the inner structure
for different structural tasks



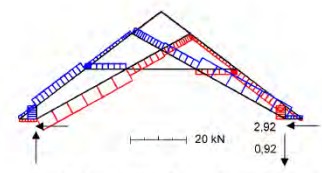
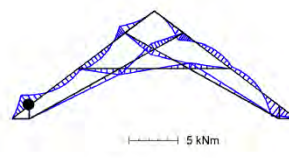
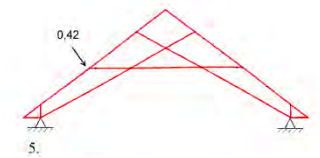
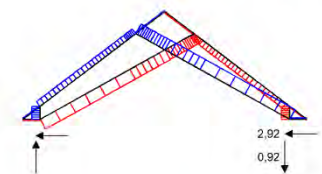
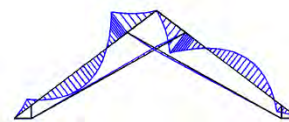
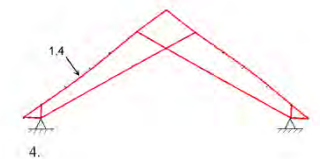
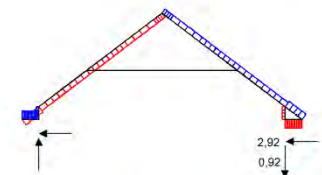
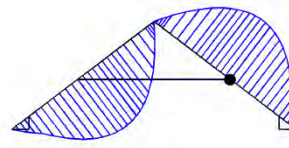
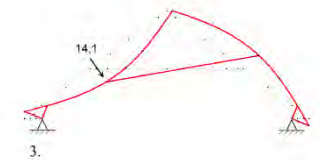
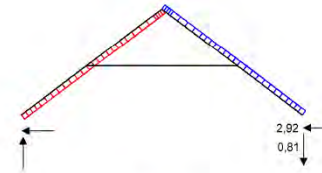
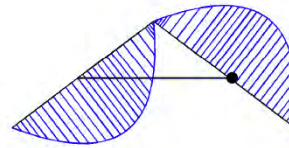
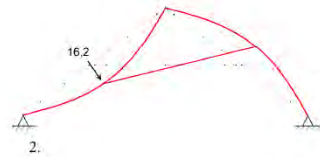
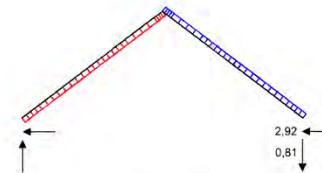
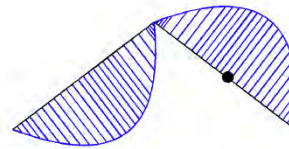
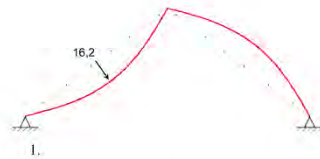
Structural tasks



**Patterns of
load paths**



Wind

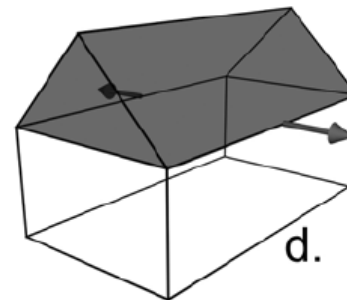
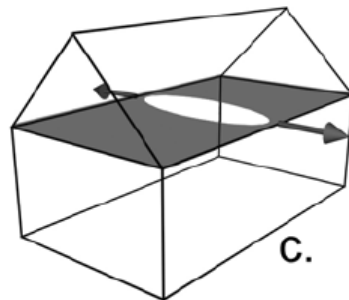
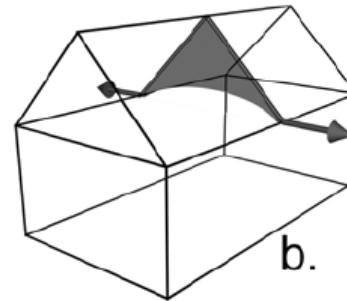
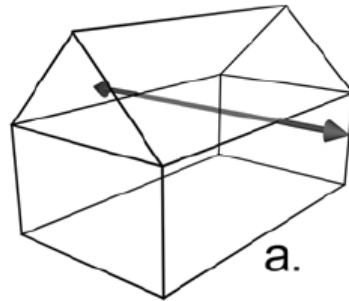


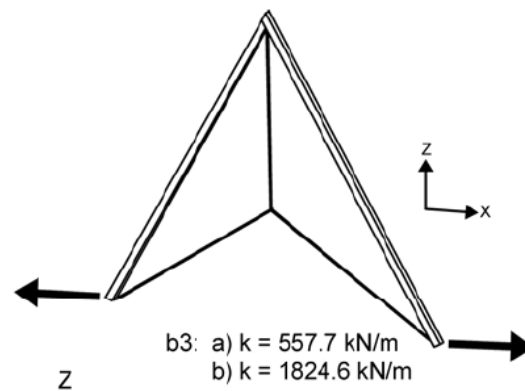
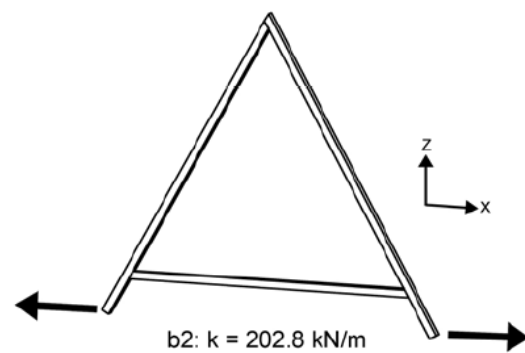
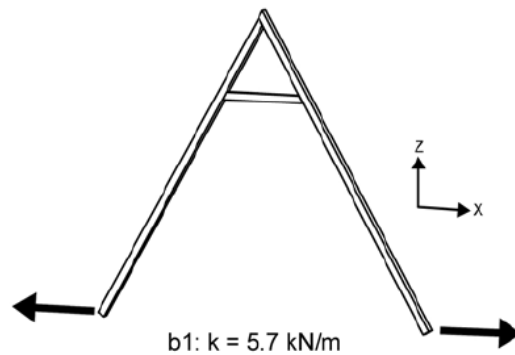
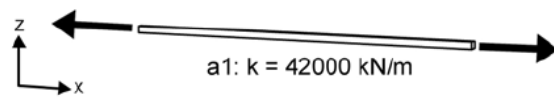
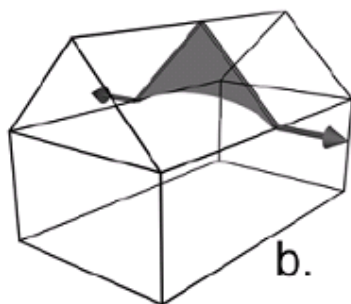
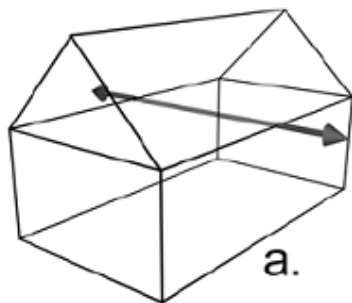
A. Deformationsfigur

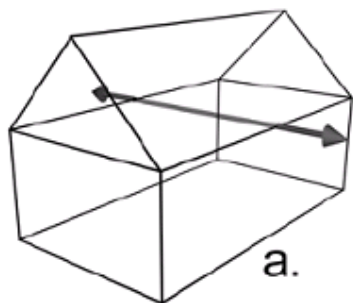
B. Böjmoment

C. Normalkrafter och stödkrafter

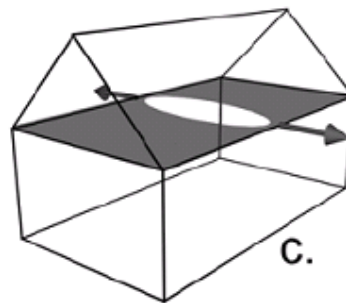
In this environment **Carl Thelin**
computes and compares efficiency
in terms of stiffness
for different internal load paths



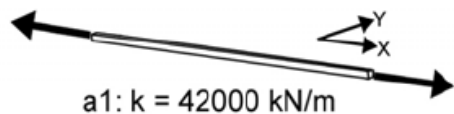




a.



c.



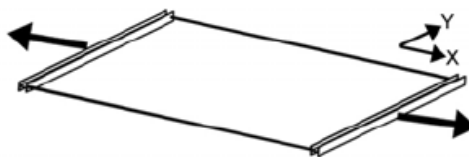
a1: $k = 42000 \text{ kN/m}$



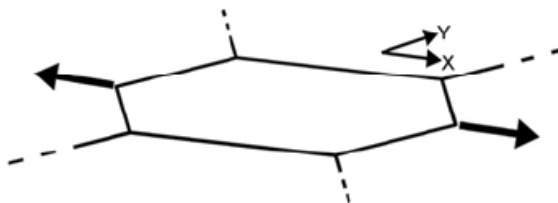
c1: $k = 400 \text{ kN/m}$



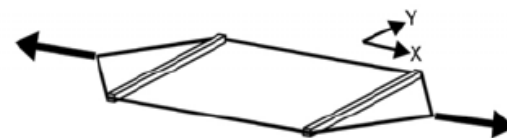
c2: $k = 65.6 \text{ kN/m}$



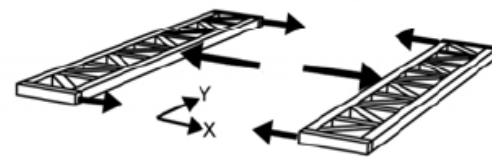
c3: a) $k = 511.9 \text{ kN/m}$
b) $k = 1009 \text{ kN/m}$



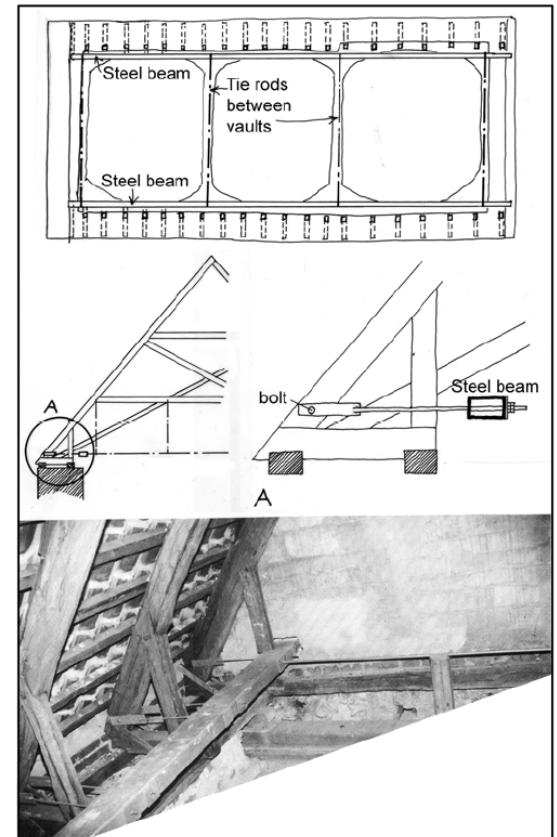
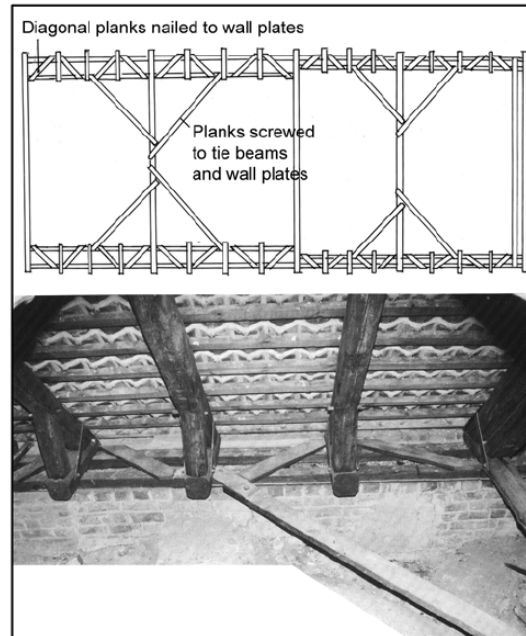
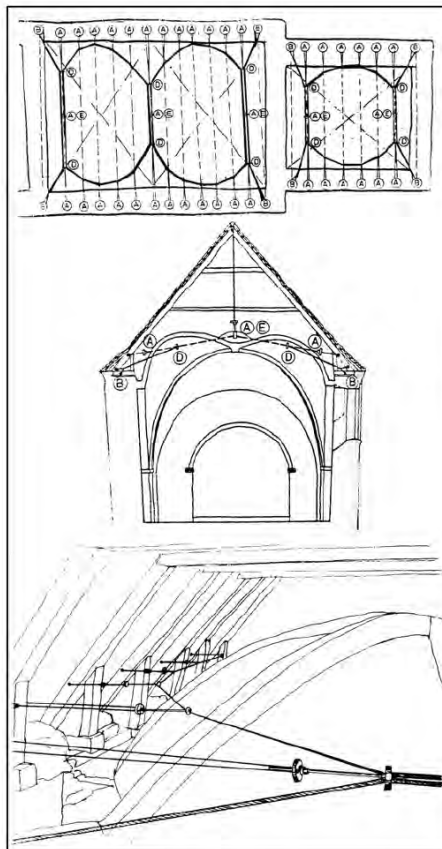
c5: a) $k = 1163.1 \text{ kN/m}$
b) $k = 4652.5 \text{ kN/m}$

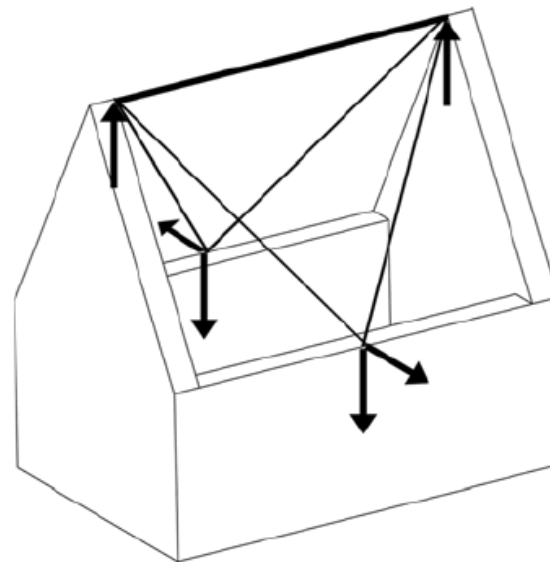
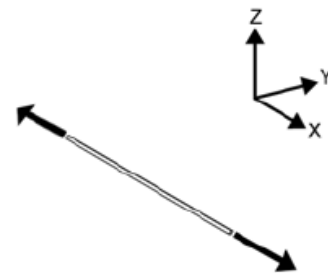
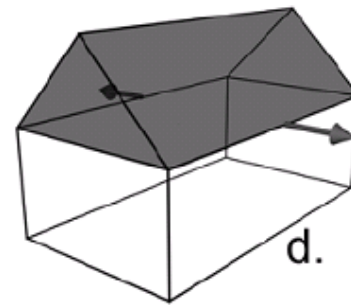
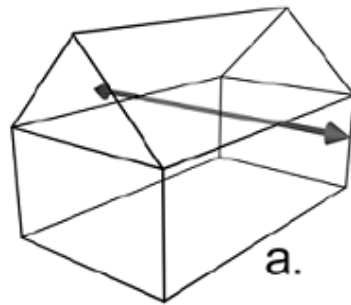


c4: a) $k = 2366.8 \text{ kN/m}$
b) $k = 7451.7 \text{ kN/m}$



c6: $k = 10703.7 \text{ kN/m}$

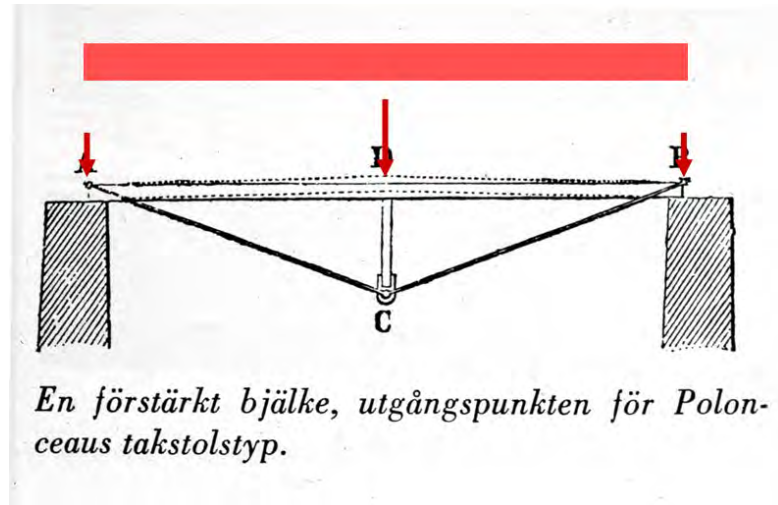




a1: $k = 42000 \text{ kN/m}$

d1: a) $k = 731.8 \text{ kN/m}$
b) $k = 2789 \text{ kN/m}$

7. Takstolen



Analysera en *Poloceau-takstol* som fackverk.

Beskriv med hjälp av pointSketch hur den fungerar i termer av inre tryck och drag.

Analysera på motsvarande sätt *S:a Catharina-takstolen* som ett fackverk. Den kommer då visa sig vara en mekanism. Ge exempel på belastning som denna takstol är känslig för (se mekanismens deformationsmönster).

Föreslå en förändring som gör takstolen mer effektiv.

~~A1 Structural Design Contest~~



8. ~~Konsten att bära en punktlast.~~

Uppgiften är att med trä, tråd och lim formge och bygga en konstruktion avsedd att bära en punktlast mitt på.

Konstruktionen ska spänna över en 90 cm bred öppning. Provbelastrning sker genom att en last hängs underifrån i en lina som ska kunna fästas vid konstruktionen.

Tänk på att konstruktionen måste klara upphängningen av linan, både vad gäller form och lokal hållfasthet.

Strukturen får väga max 130 gram.

Två konstruktioner per ritsal.
Redovisning torsdag den 14/3.

Föreläsning onsdag den 11/3
med start 9.00



Vecka 10

- Onsdag 10/3 09.00 - 9.45 Zoom OBS tiden!
Föreläsning: Konsten att bära en punktlast
10.00 - 11.45
Morten Lund: Broar - arkitektur och konstruktion
- Torsdag 11/3 08.00 - 17.00 Ritsalar/ Eget arbete
Sammanställning av Lärobok
- Fredag 12/3 BYGGDAG RESERV [Zoom](#)
08.00 - 17.00
Sammanställning av Lärobok

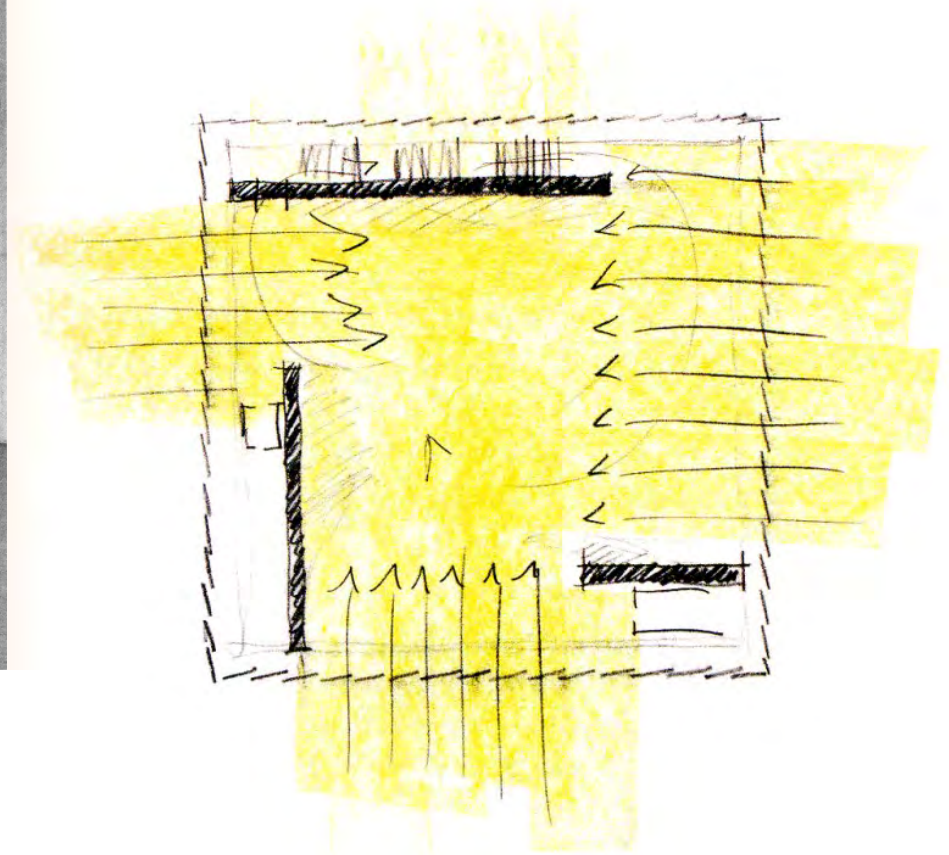
Skivan och pelaren



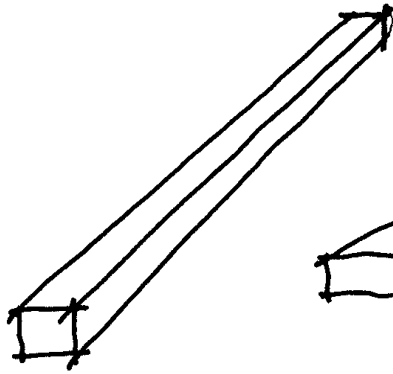
Skivan
för stabilisering
för horisontella laster



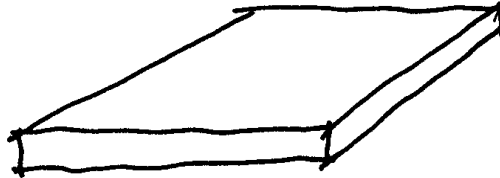
KunsthauB BregenZ
Peter Zumthor



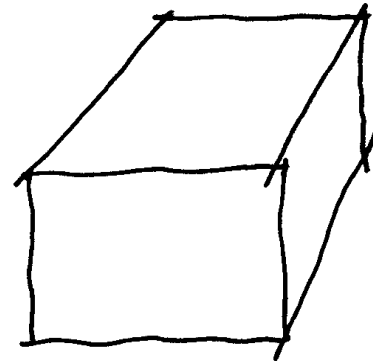
Strukturdelar sorterade efter kroppens dimensioner



1-dim
Stång
Balk

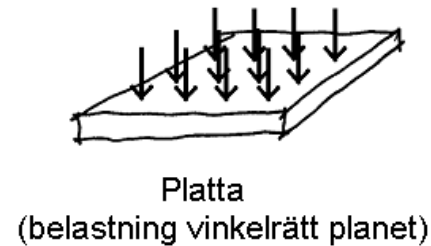
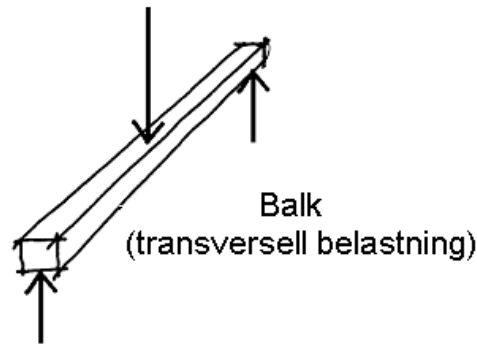
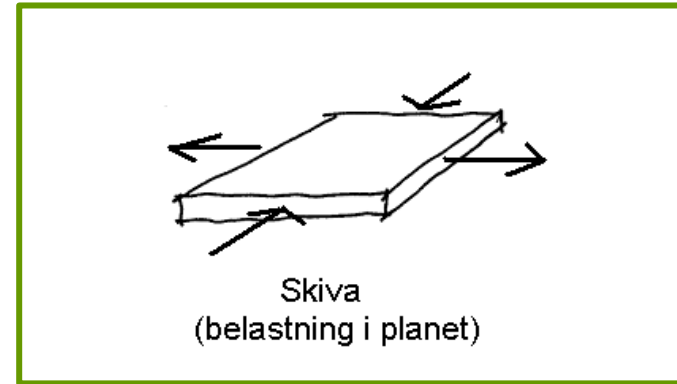
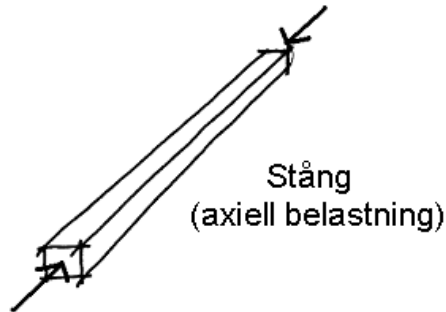


2-dim
Skiva
Platta

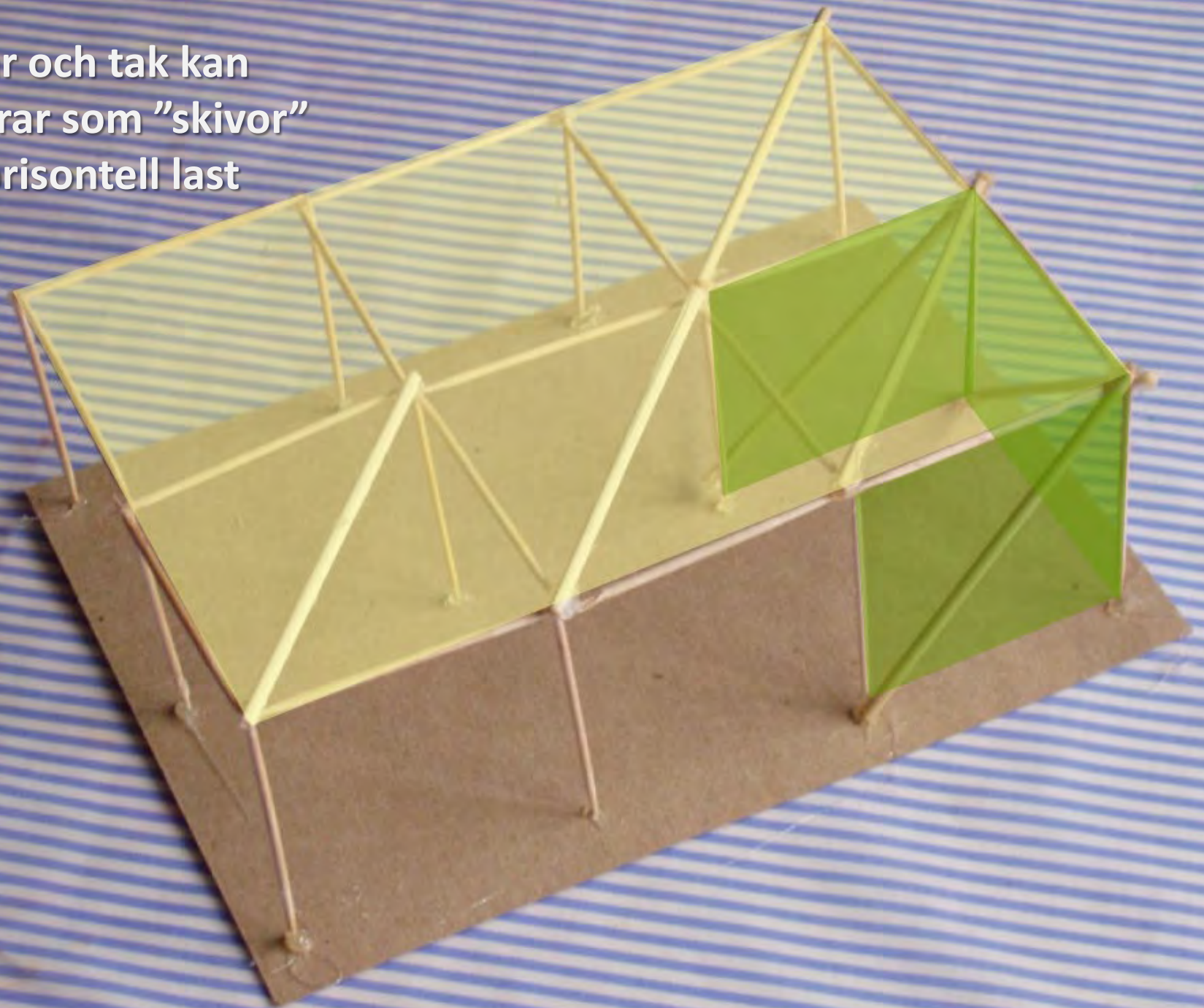


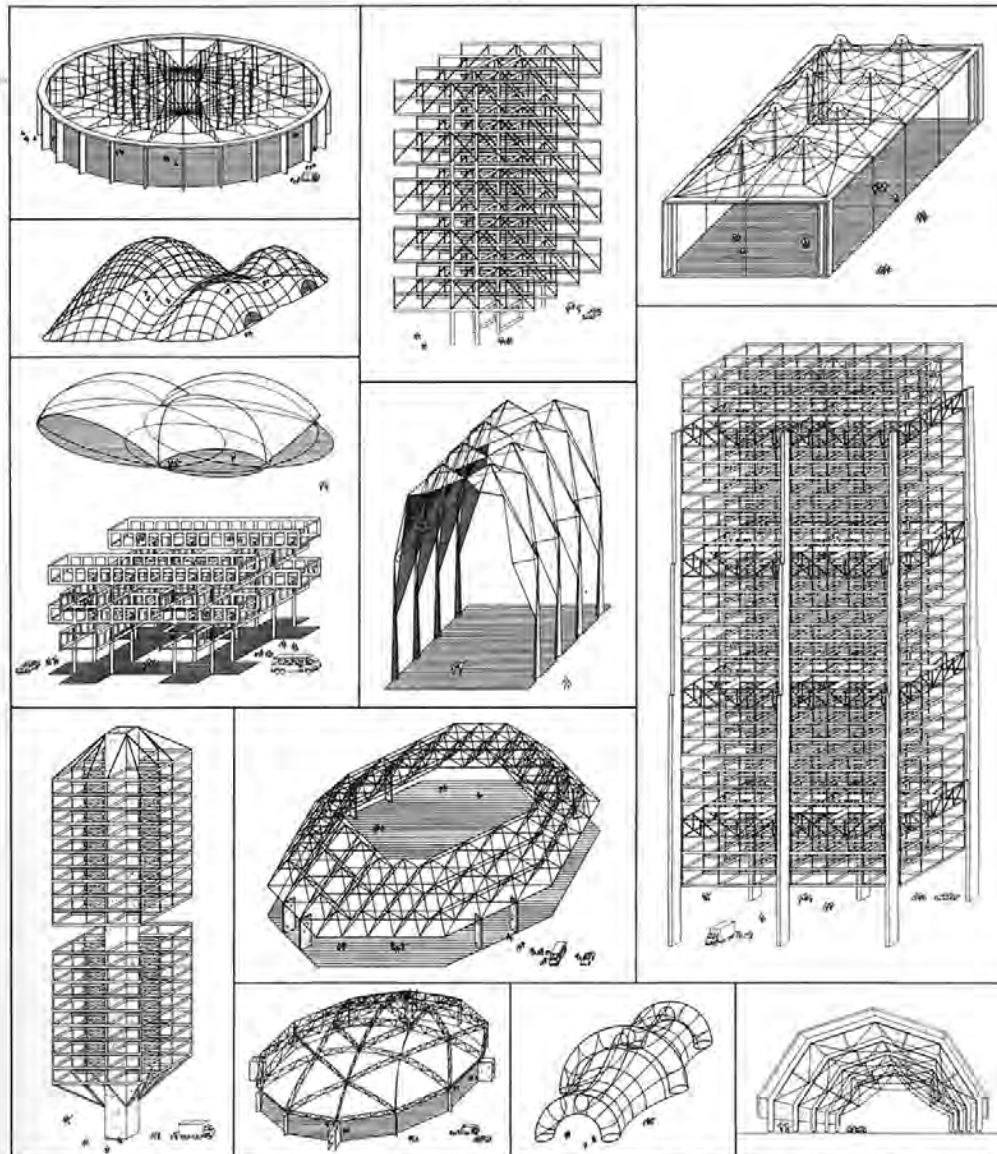
3-dim
Solid

Konstruktionselementens verkningssätt (form + belastning)



Väggar och tak kan
fungerar som "skivor"
för horisontell last





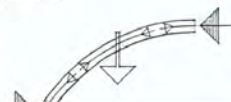
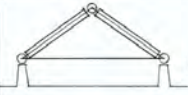
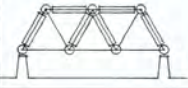
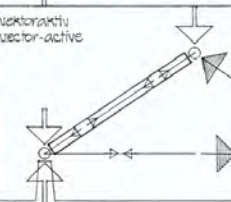
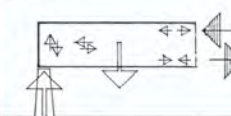
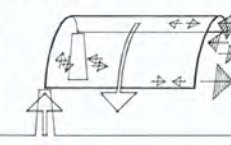
Strukturer sorterade efter hur form möter last

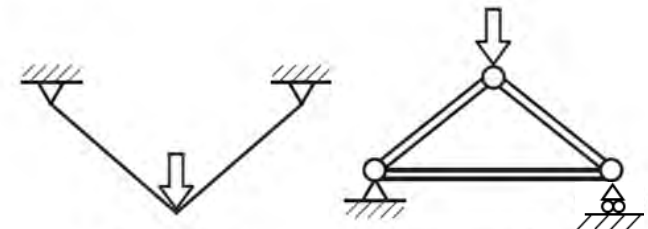
Heino Engel
Structural Systems

Formaktiva strukturer
(linan, bågen)
Vektoraktiva strukturer
(fackverket)
Snittaktiva strukturer
(balken, ramen)
Ytaktiva strukturer
(skivan)

Einteilung der Tragsysteme im Bauen

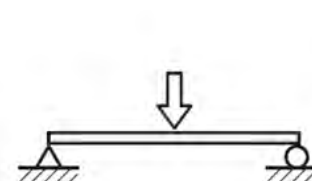
Classification of structure systems in building

Kriterium Criterion	Prototyp prototype	Kräfte forces	Merkmale feature	Mechanik der Kraftumleitung mechanics of redirection of forces
1 FORM	 Stützboogen funicular arch Hängeseil suspension cable  Kreisring circular ring Ballon balloon	Druck oder Zug compression or tension	Stützlinie thrust line Kettlinie catenary Kreis circle	formaktiv form-active 
2 VEKTOR VECTOR	 Dreiecksträger triangular truss  Fachwerkbügel trussed beam	Druck und Zug compression and tension	Dreieck- verbund triangu- lation	vektoriell vector-active 
3 QUER- SCHNITT CROSS SECTION	 Balken beam  Rahmen frame  Platte flat slab	Biegung Schnittkräfte bending section forces	Querschnitt- profil sectional profile	schnittaktiv section-active 
4 FLÄCHE SURFACE	 Schale plate gefaltete Platte folded slab  Zylinderschale cylindrical shell	Membran- kräfte membrane forces	Flächen- form surface shape	flächenaktiv surface-active 

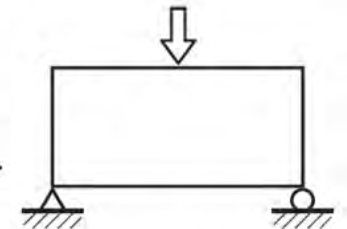


Formaktiv

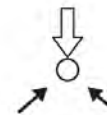
Vektoraktiv



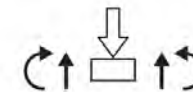
Snittaktiv



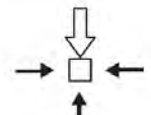
Ytaktiv



Partikeljämvikt

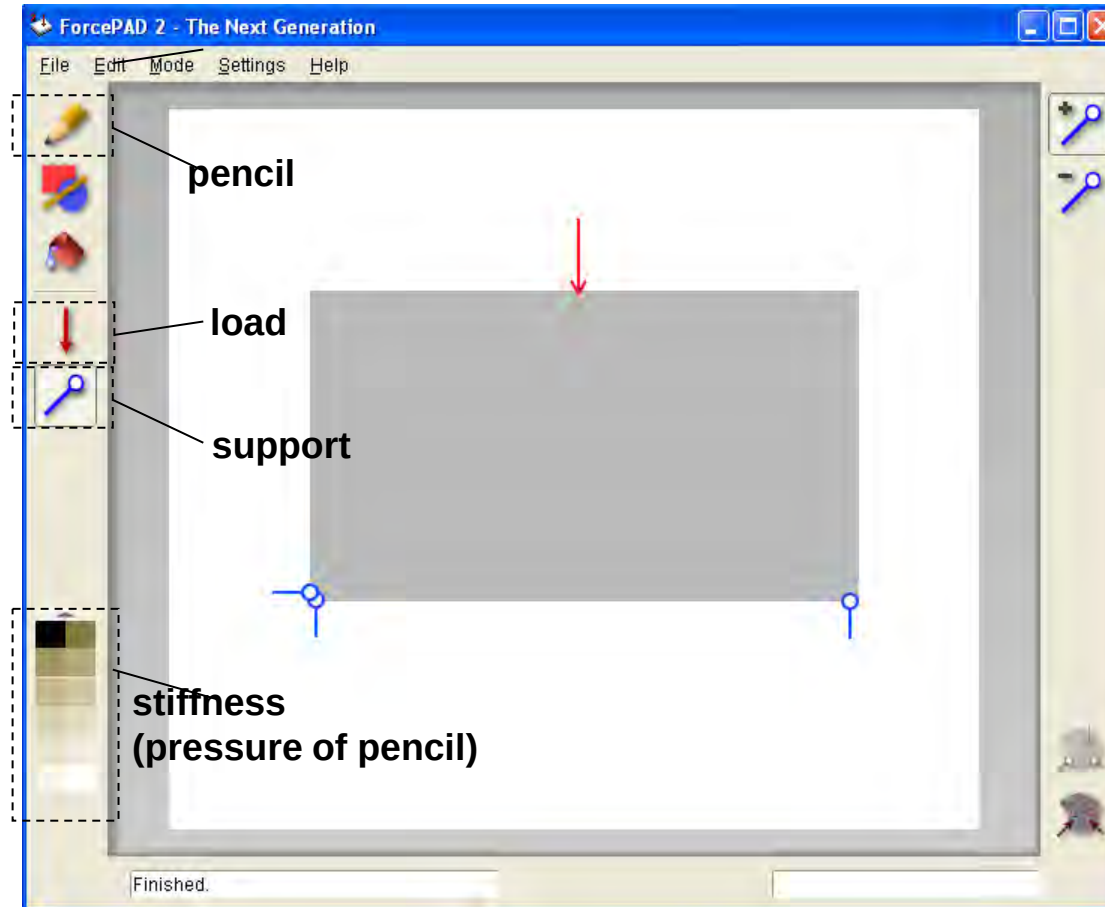


Stelkroppsjämvikt



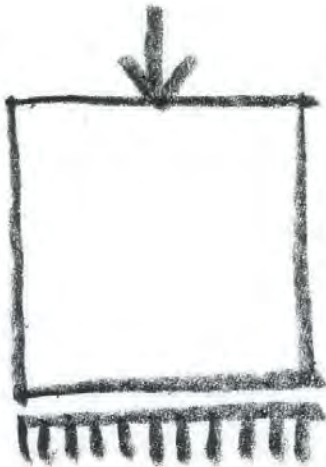
Kontinuumjämvikt

Force PAD – analyserar skivor

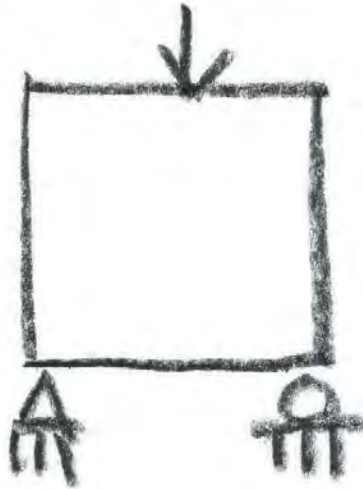


Skivan

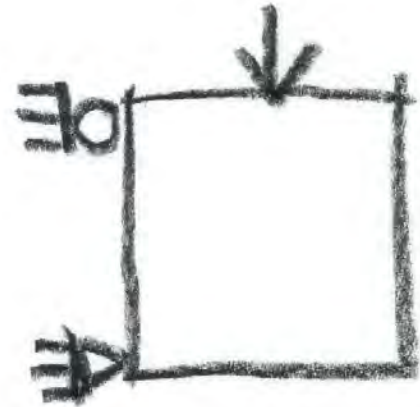
och tre grundläggande uppgifter



Lyfta upp



Spänna över



Nå ut

Jämför med

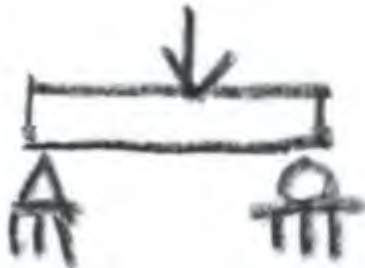
Pelaren

Fritt upplagda balken

Konsolen



Lyfta upp



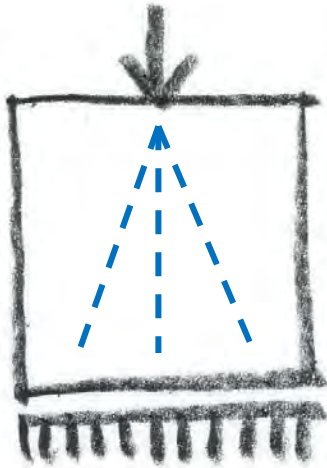
Spänna över



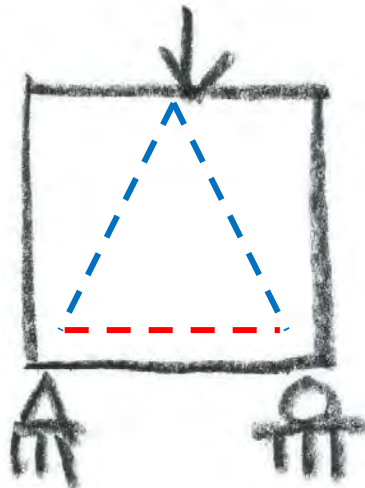
Nå ut

Skivan - 2D form

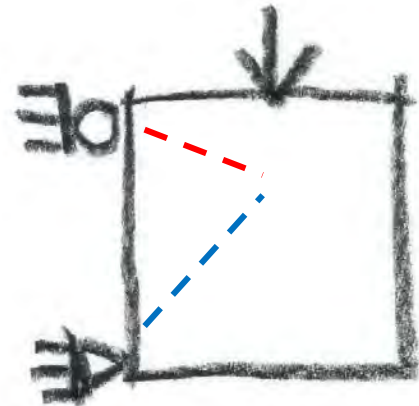
synliggör tre
grundläggande kraftmönster



Lyfta upp



Spänna över



Nå ut

Samma mönster i 1D form

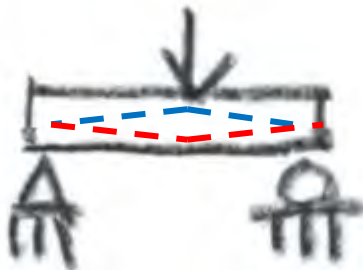
Pelaren

Fritt upplagda balken

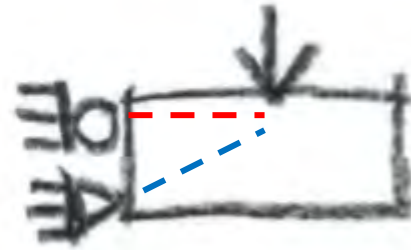
Konsolen



Lyfta upp

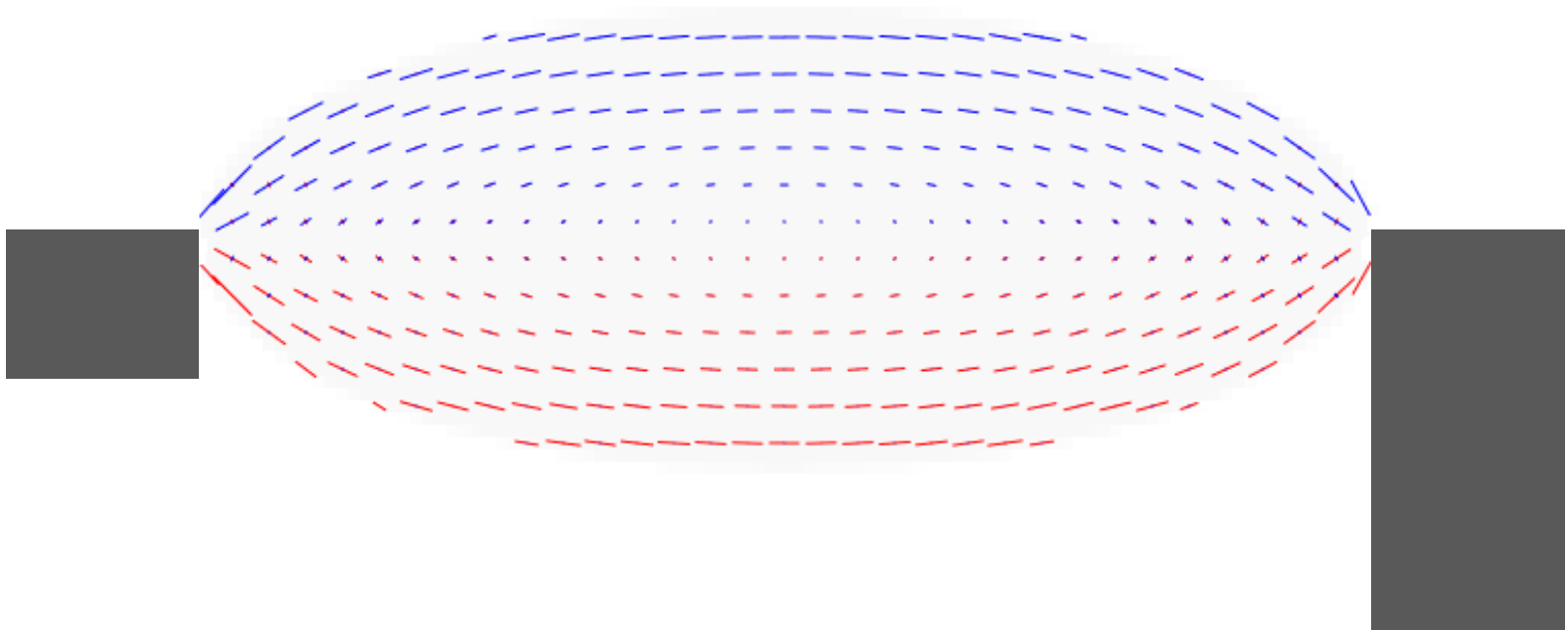


Spänna över

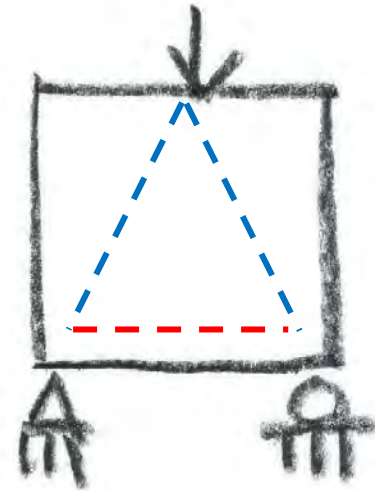
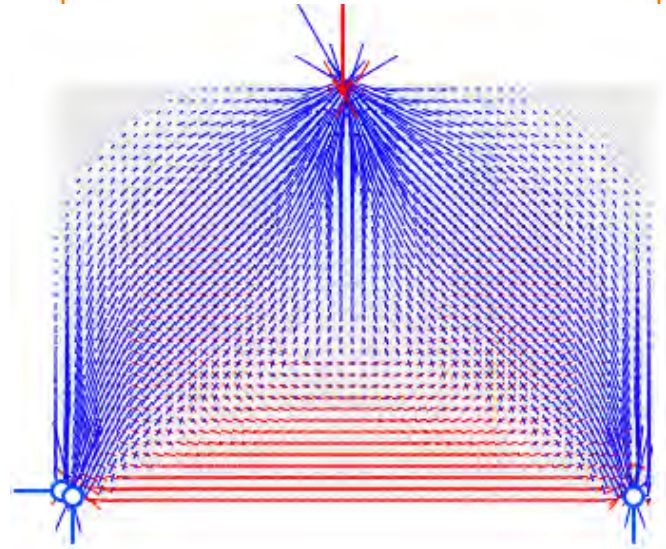
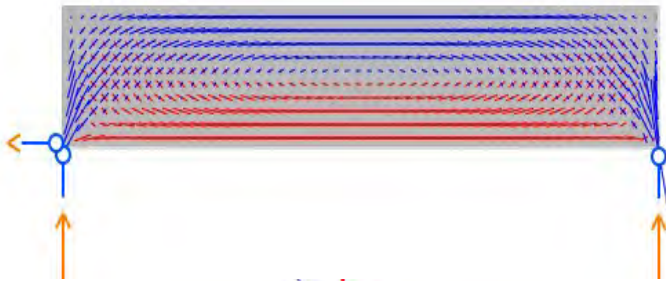


Nå ut

Kraftmönster – spänna över

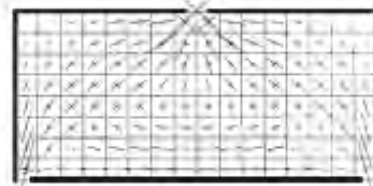
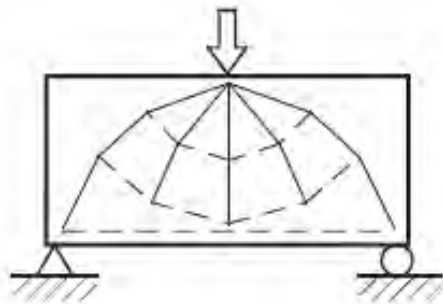
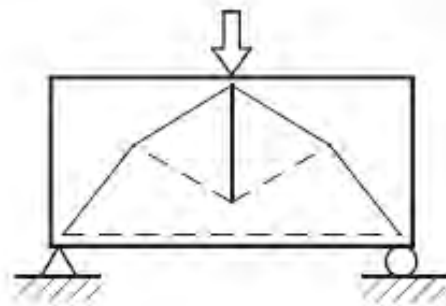
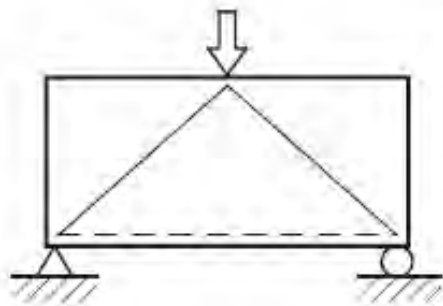
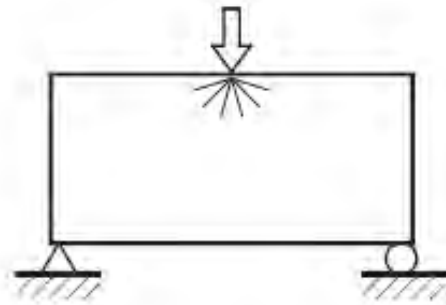
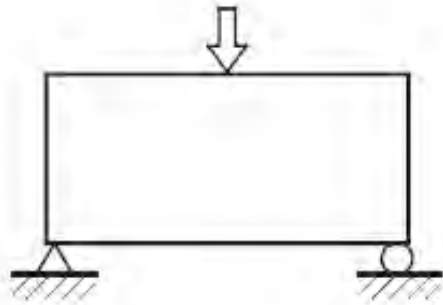


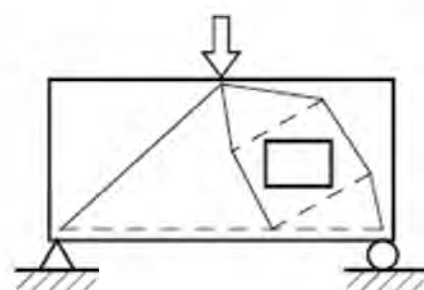
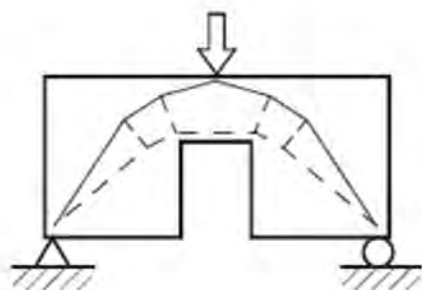
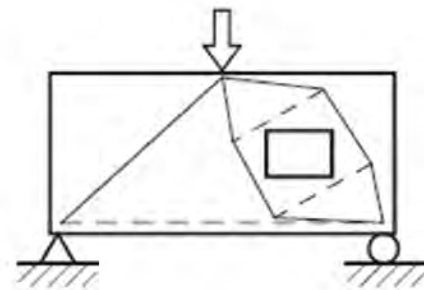
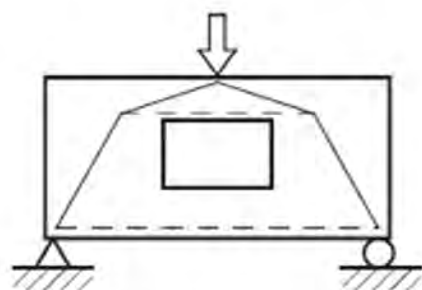
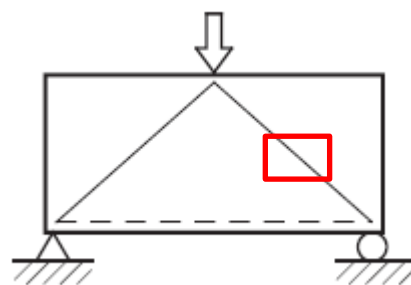
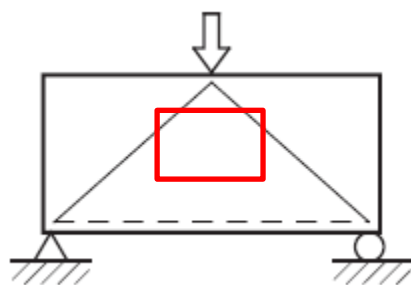
Kraftmönster – spänna över



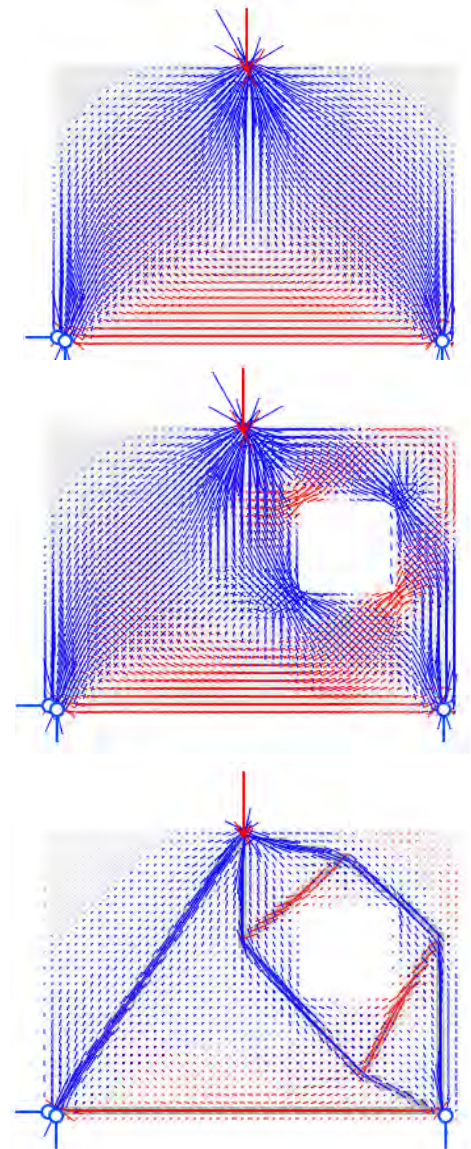
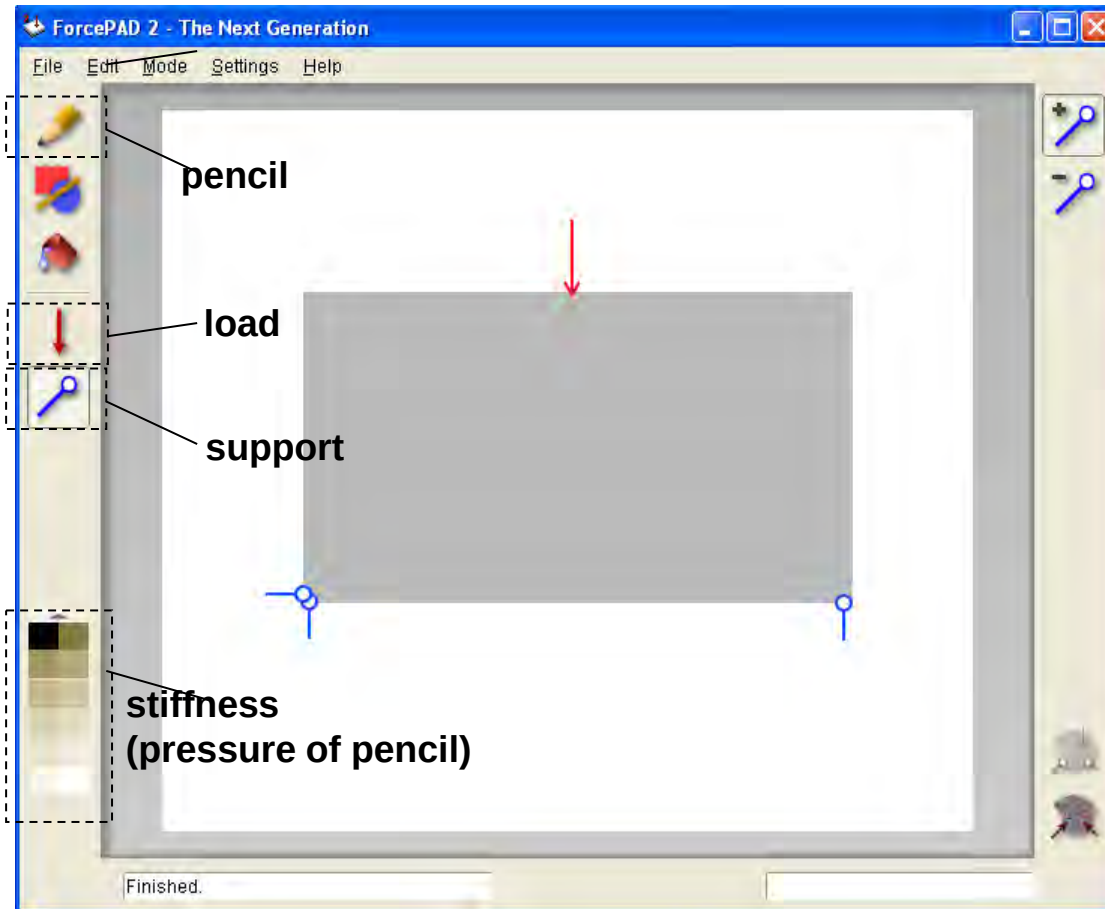
Spänna över

Från kedjekurvor och trycklinjer till spänningsmönster i en skiva

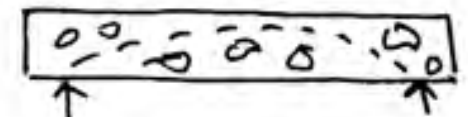
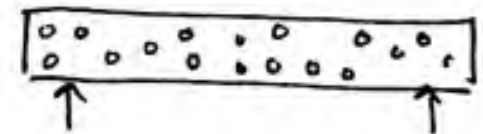
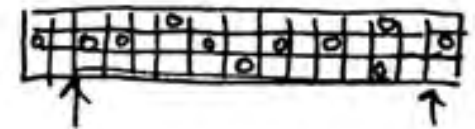
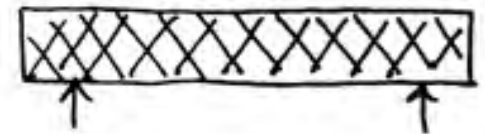
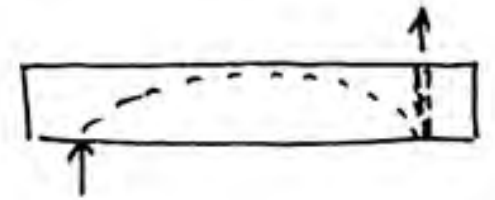




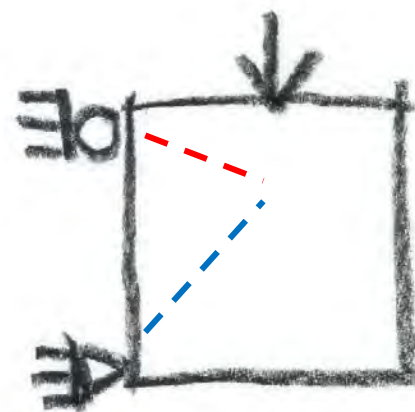
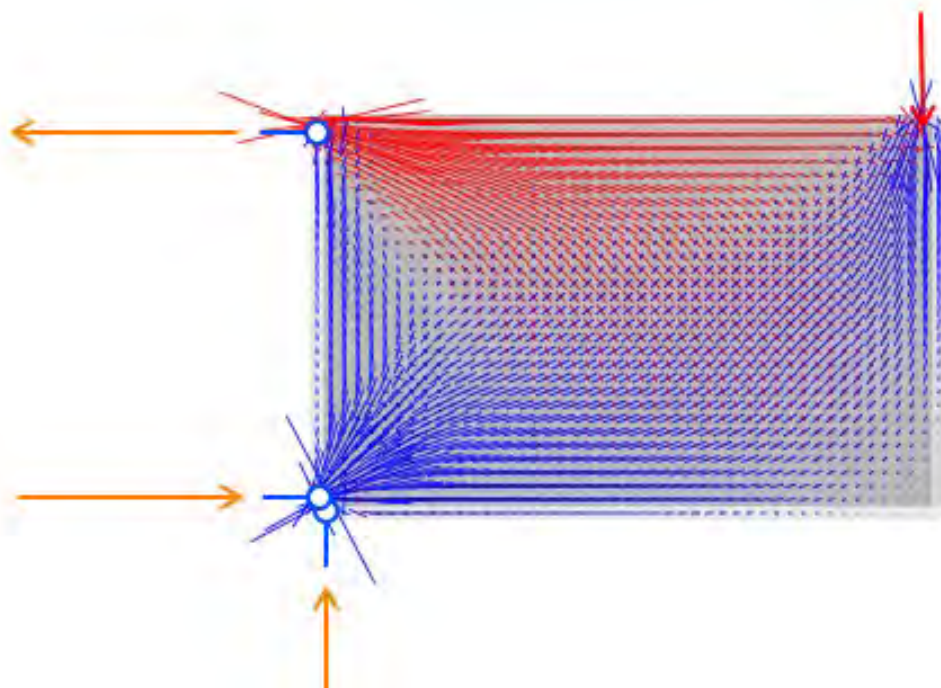
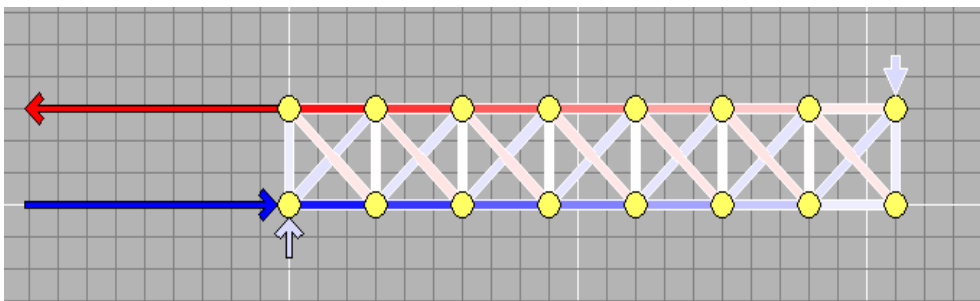
Force PAD – att skissa i ett kraftfält



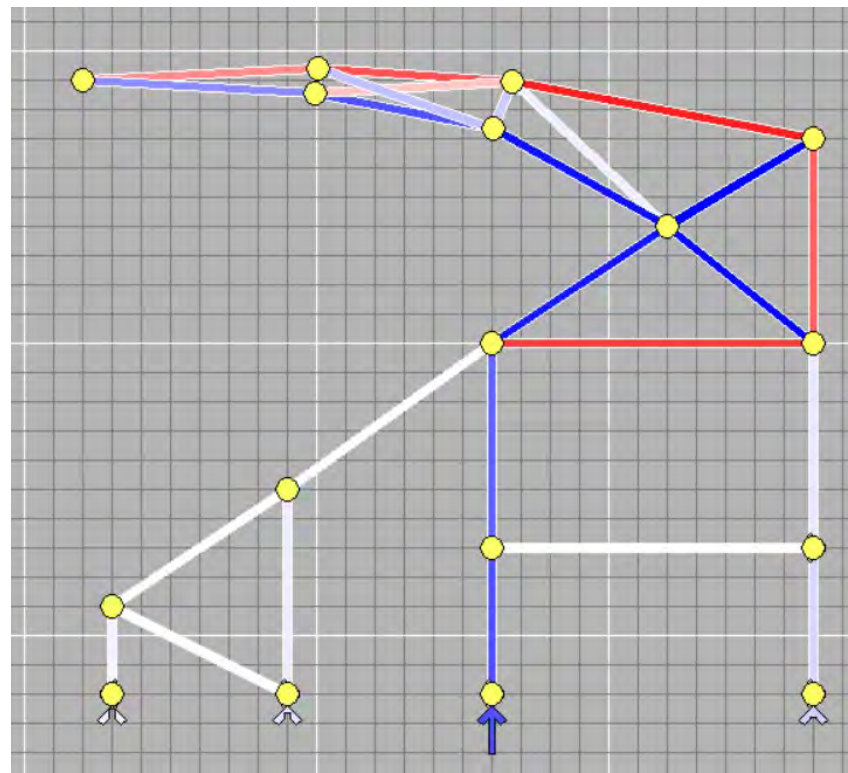
Skivan som "fritt upplagd balk"



Kraftmönster – nå ut



Nå ut

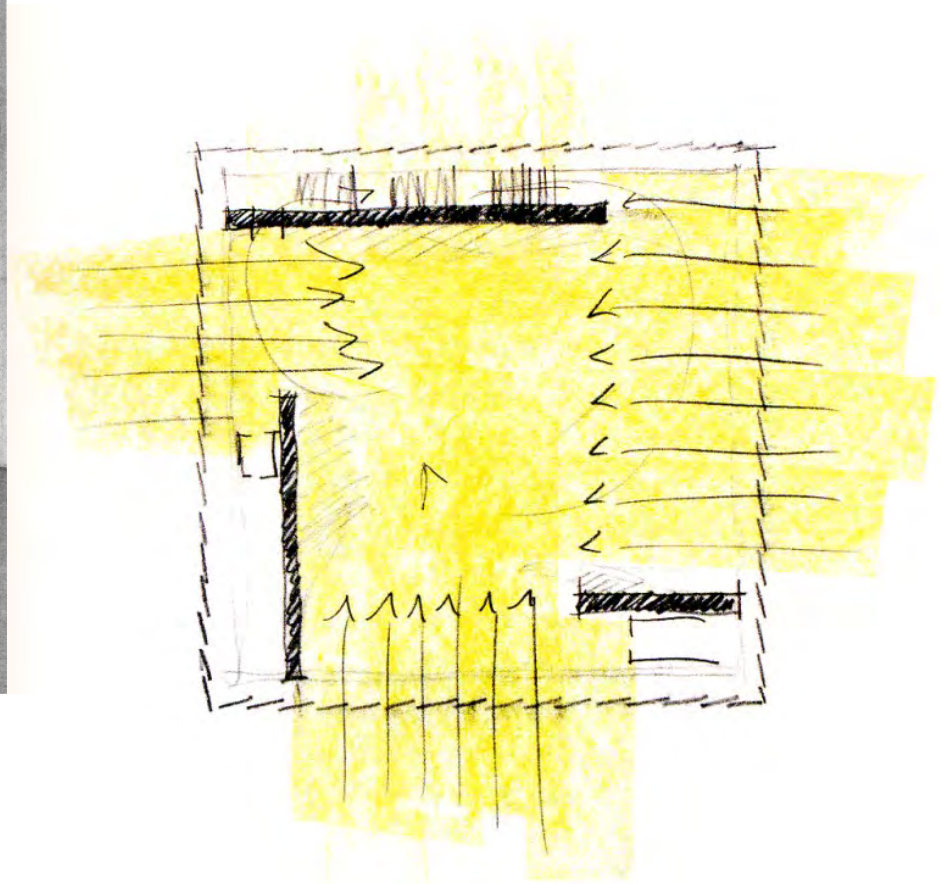


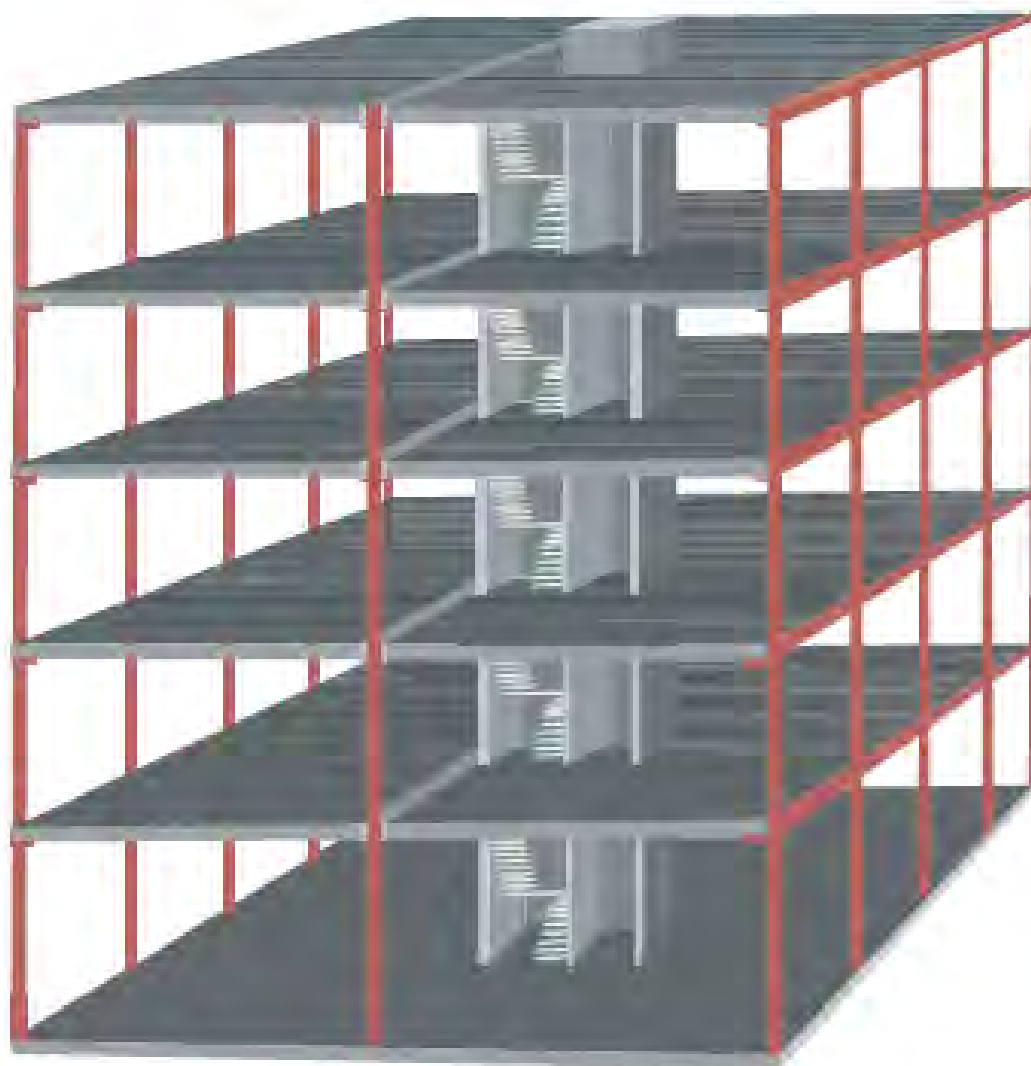
Piero Luigi Nervi
Florence Municipal Stadium
1930-32 and 1950-51



Skivan som "konsolbalk"

till exempel för
horisontell stabilisering

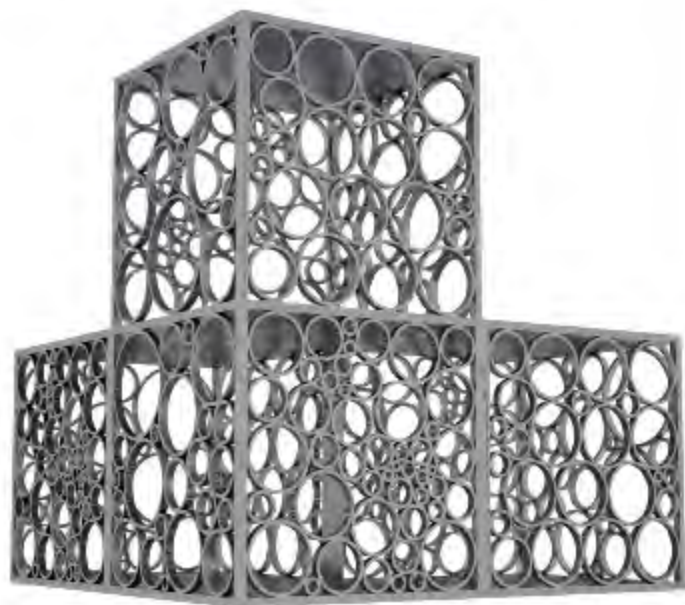


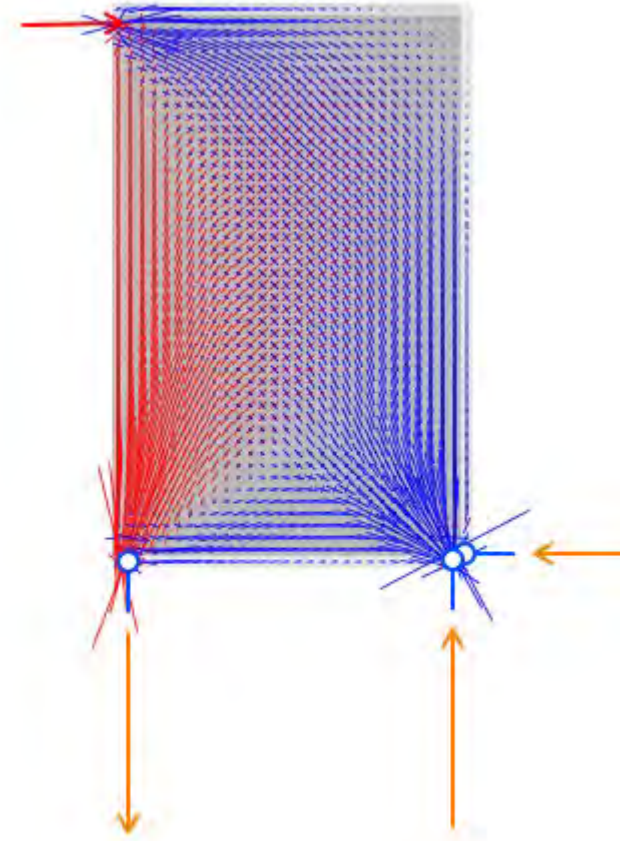
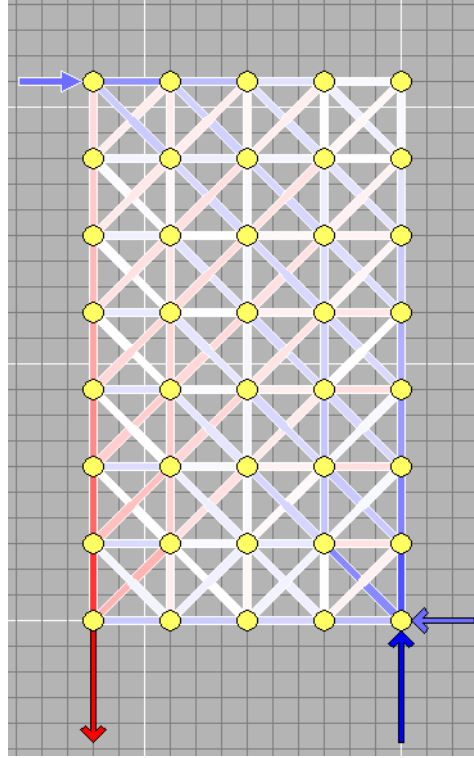
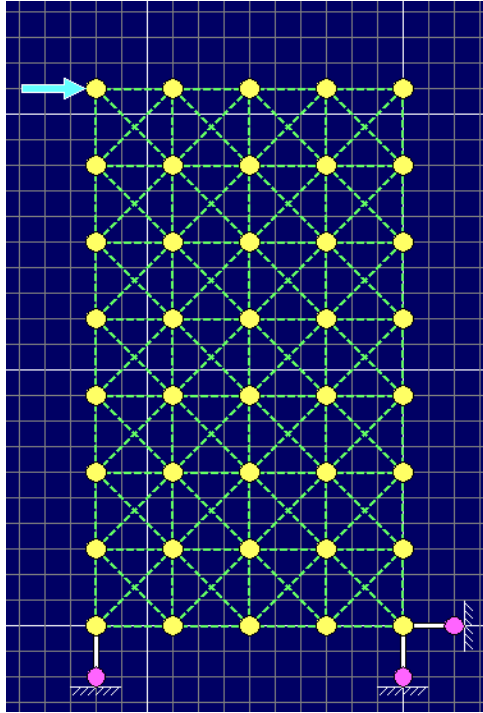


3. Stålstomme med betongbjälklag.

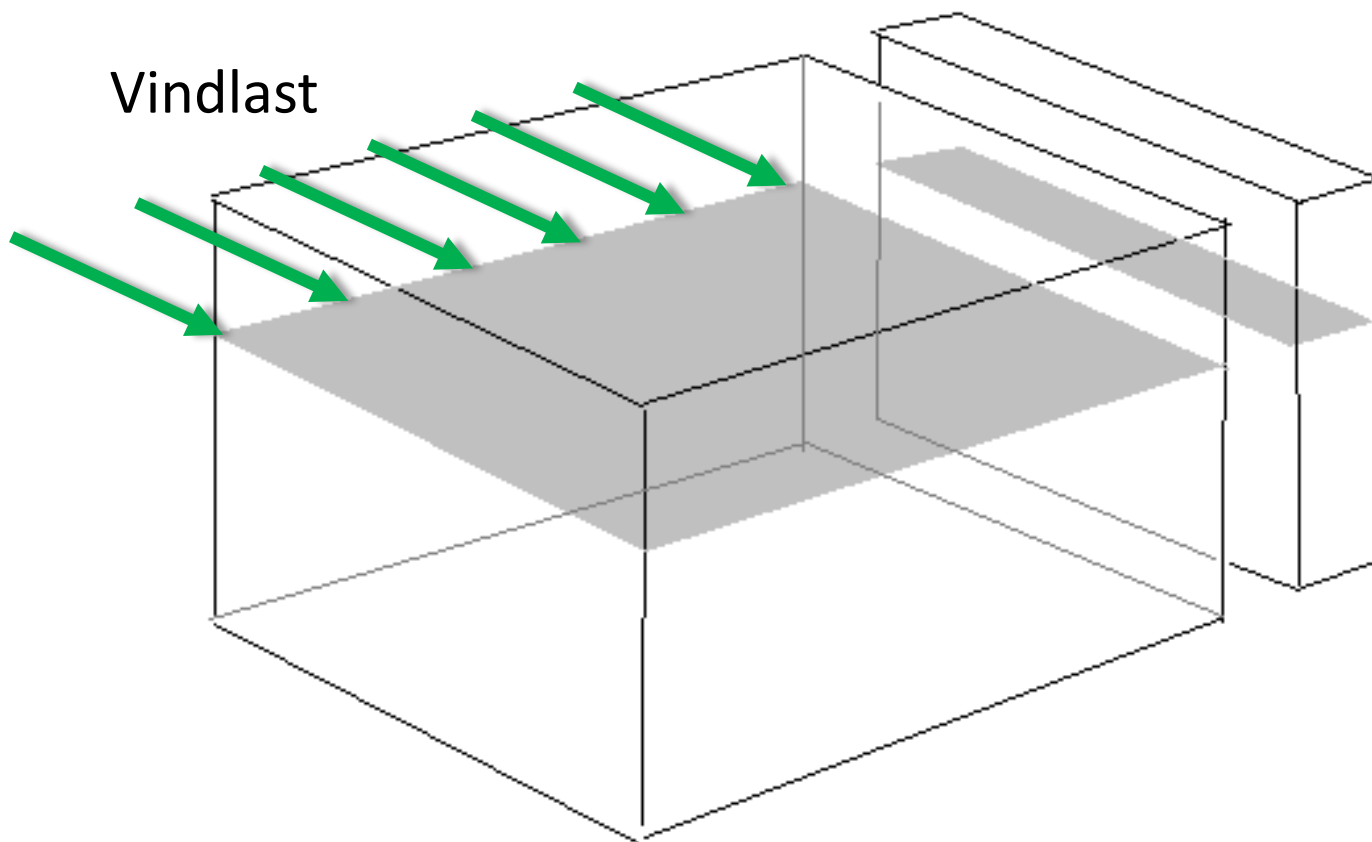
Strängbetong levererar också Basbyggnaden i ett utförande där bjälklagen bärs upp av pelare och balk i stål.

Tumregel
 $H/L = 1/10$



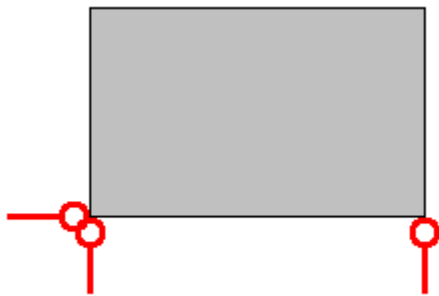


Ett bjälklagsplan betraktat som en skiva

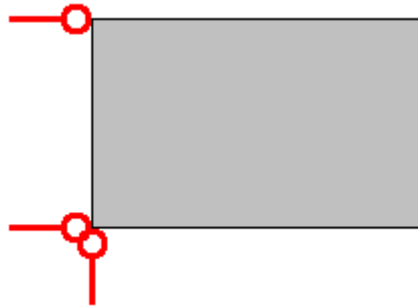


Bjälklagsplanet i sig själv är en **inre stabil** skiva – **yttre stabilitet** ?

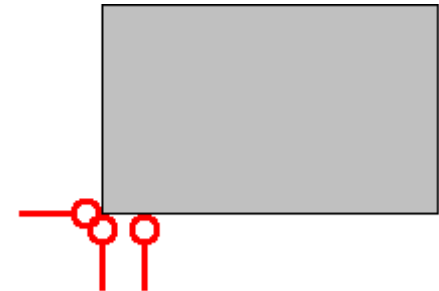
För stabilitet behövs minst **tre låsta frihetsgrader**
två vinkelräta riktningar och en rotation



Stabiliserande
ytterväggar

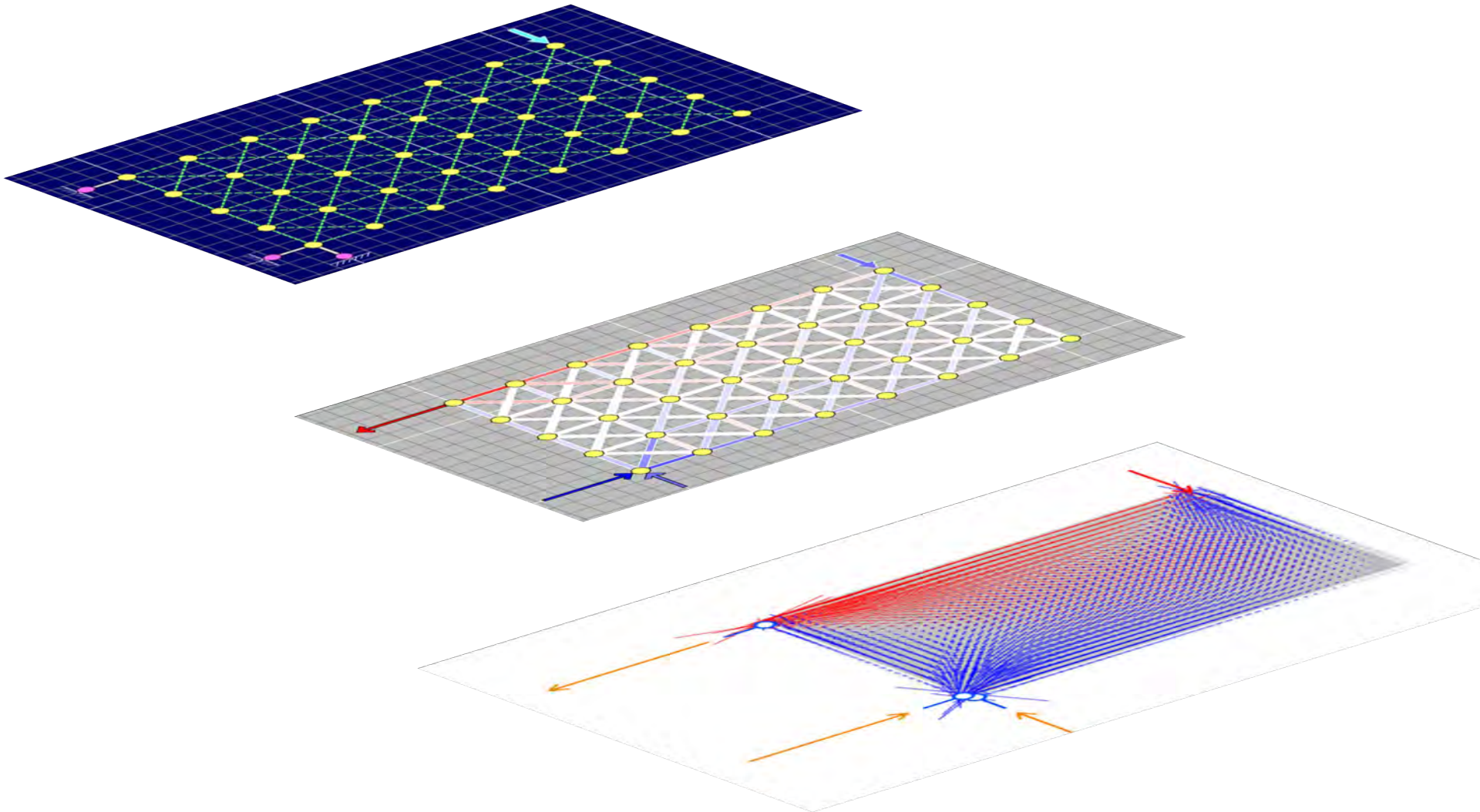


Stabiliserande
ytterväggar

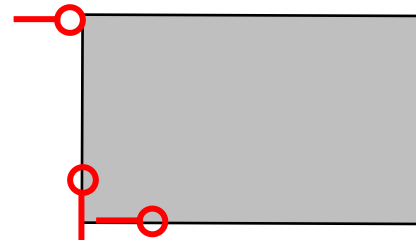
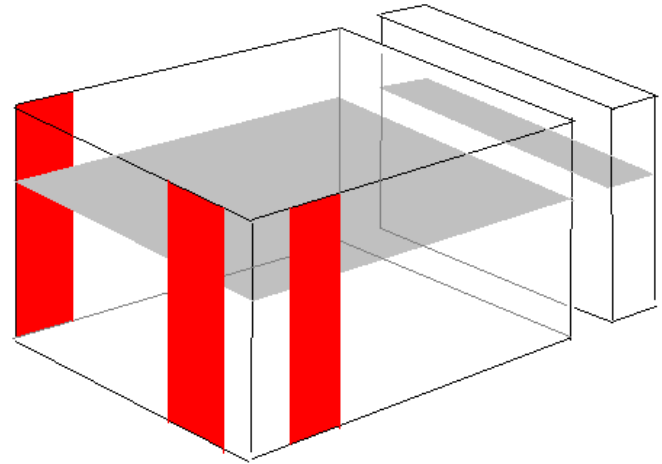
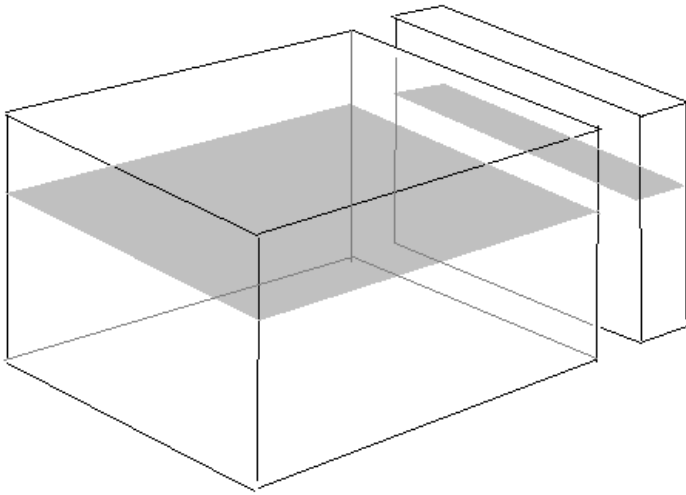


Stabiliserande
trapphus

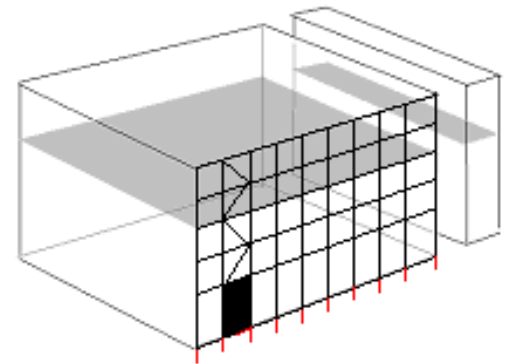
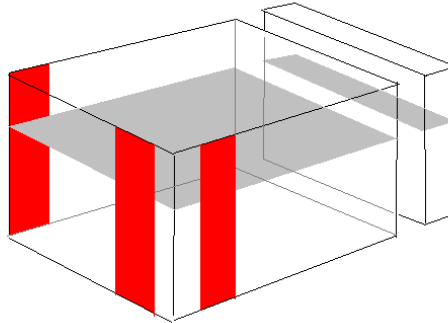
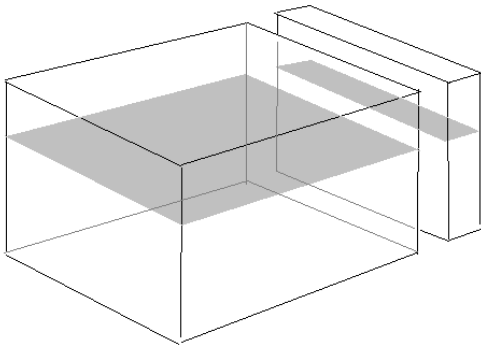
Bjälklagsplan modellerat i pointSketch och i ForcePAD

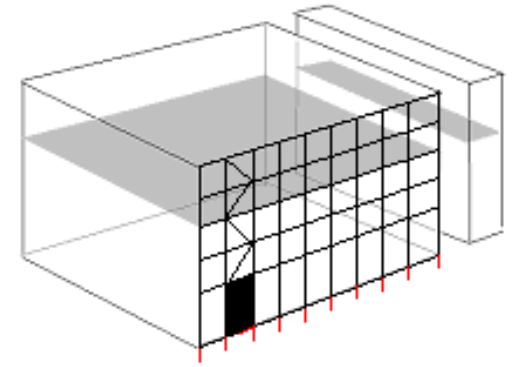
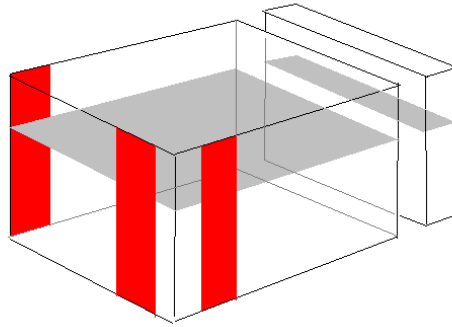
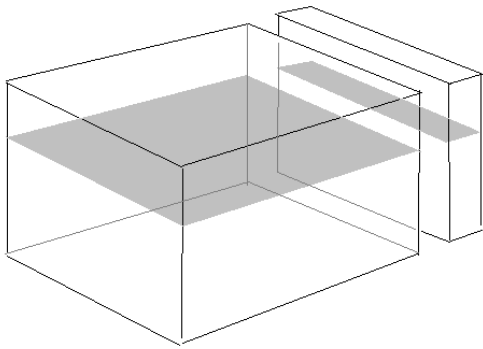


Stabiliserande väggar

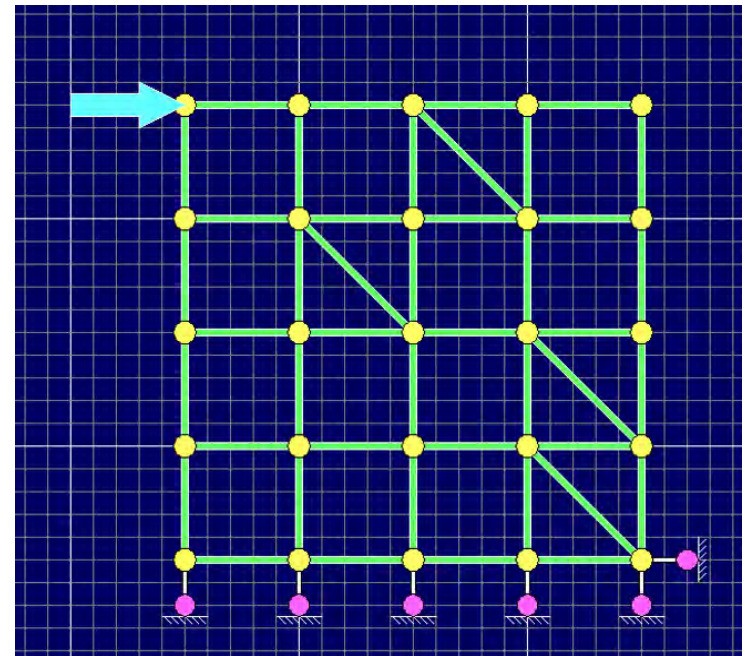


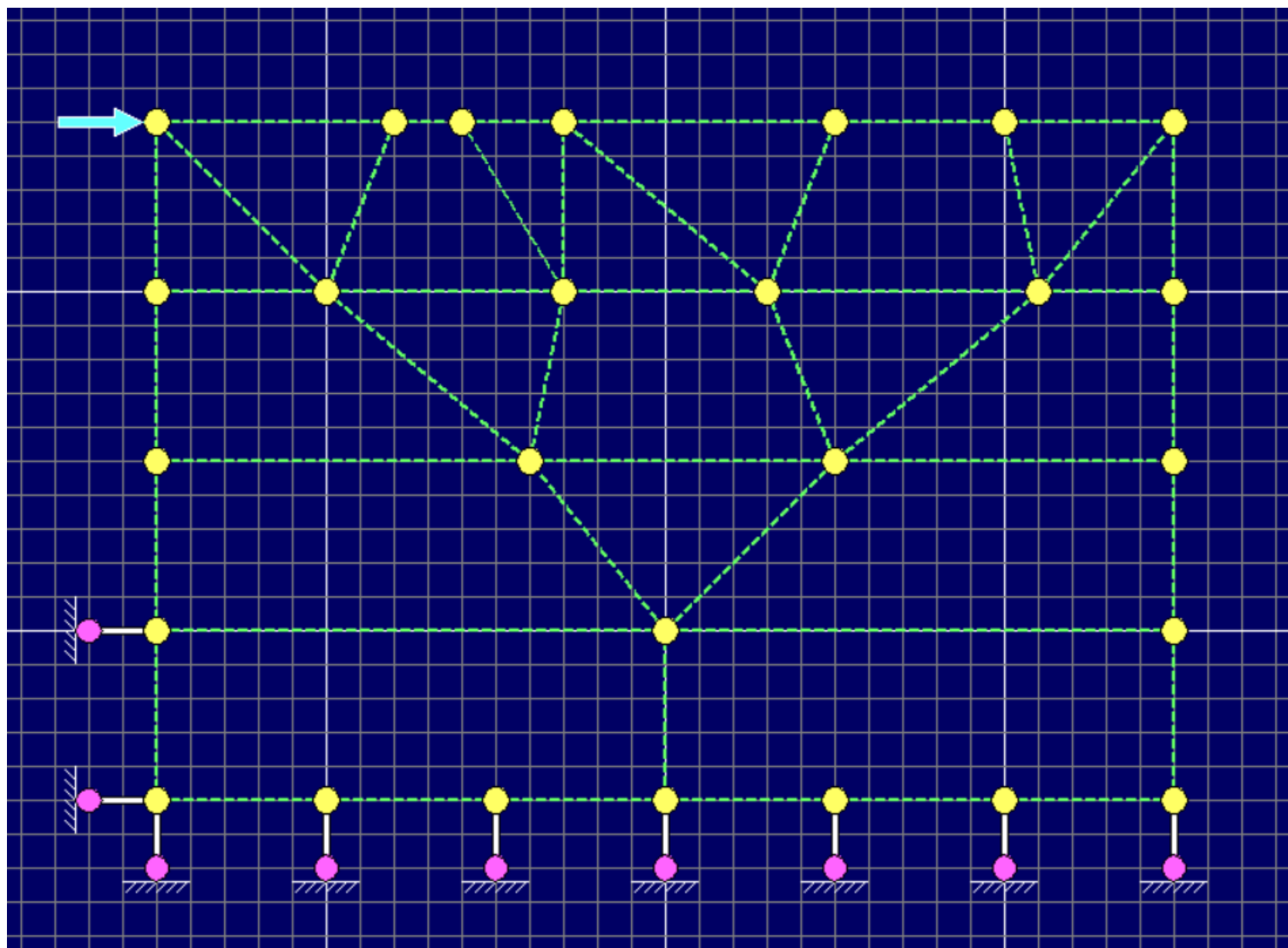
Stabiliserande väggar – samlade i begränsade zoner ...



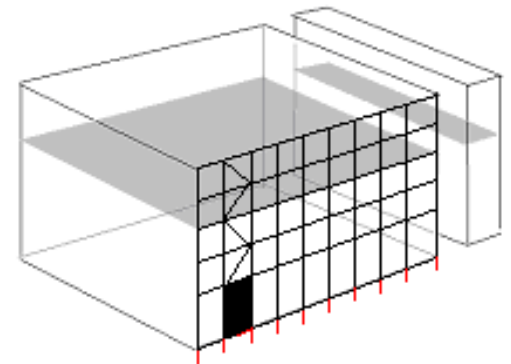
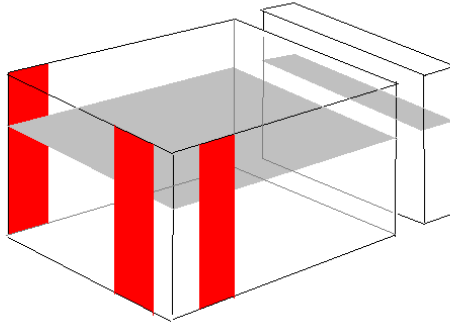
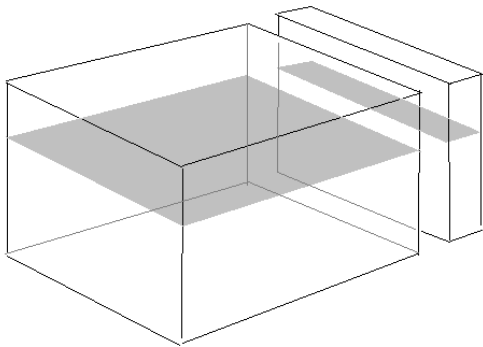


**... eller utspridda
över hela väggytan**

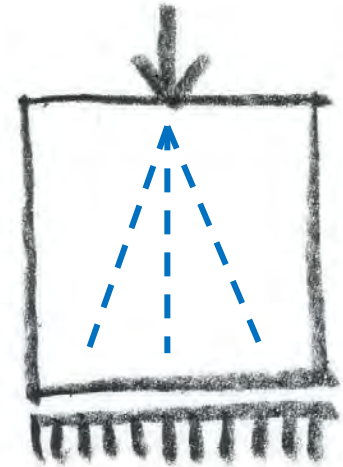
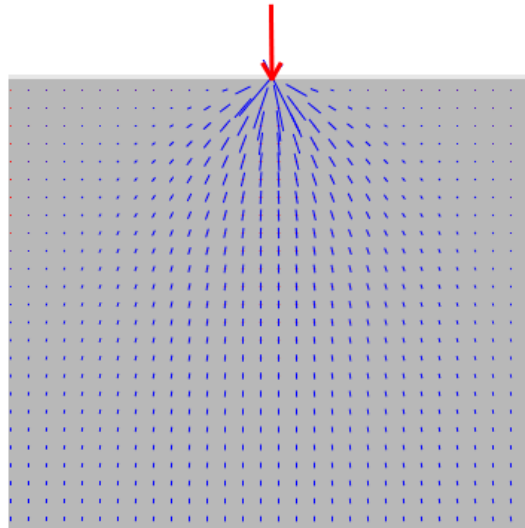
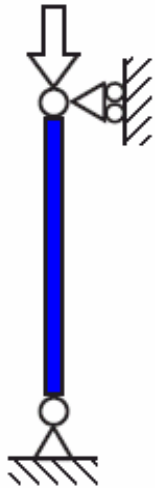




Yttre stabilisering av flera horisontella bjälklagsskivor i samma plan

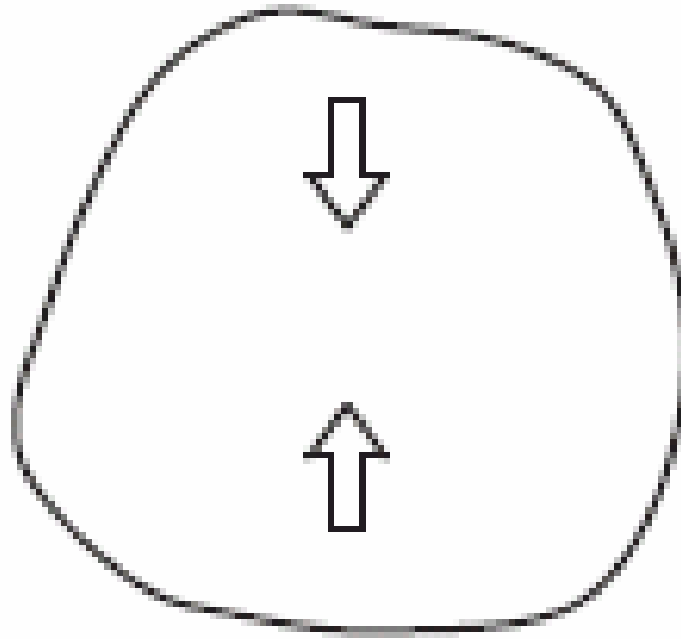
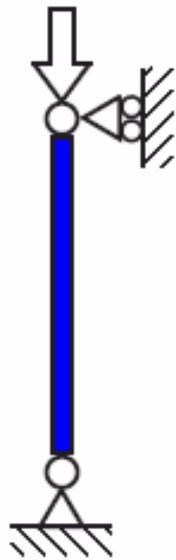


Kraftmönster – lyfta upp



Lyfta upp

Pelaren sedd som en skiva



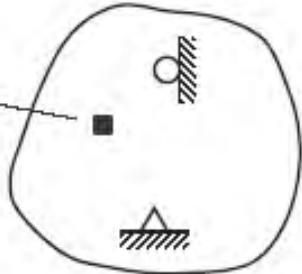
Samma materialstyvhets E i hela skivan

materialstyvhets

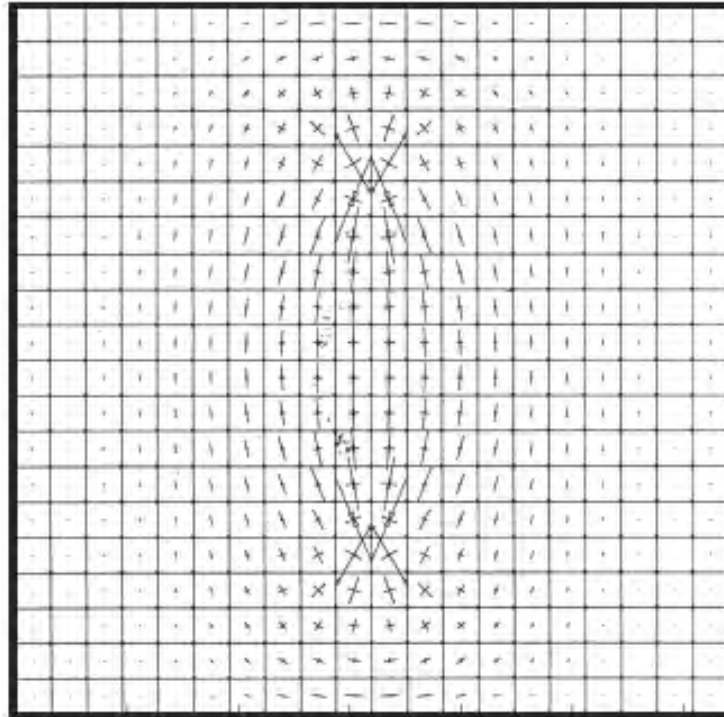
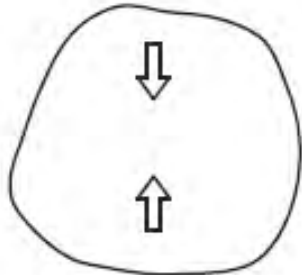


och

yttre stöd

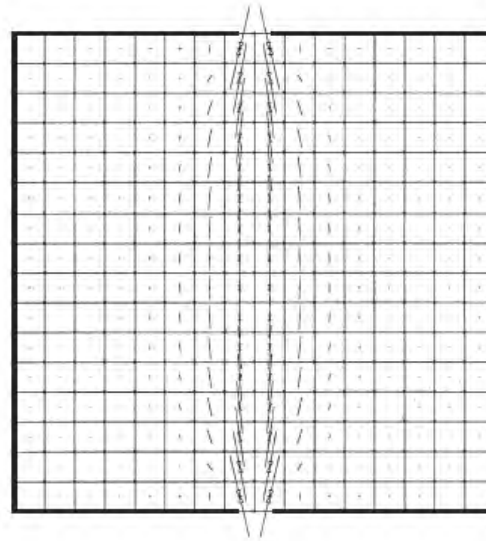
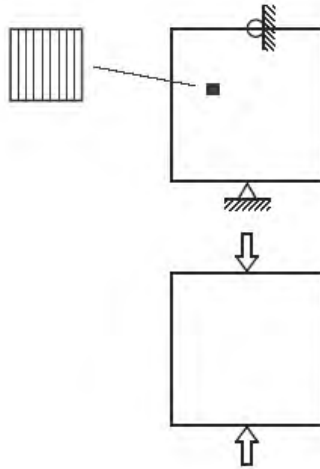


yttre last och
yttre stödkraft

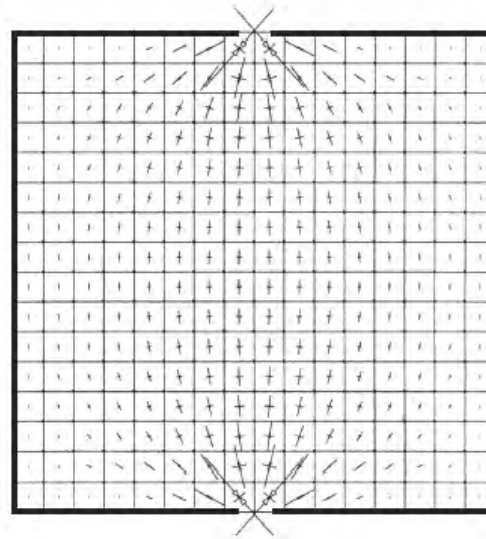
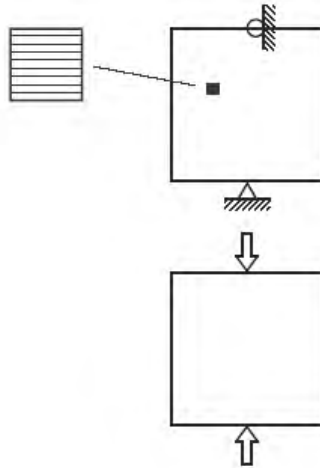


Olika materialstyvhet E i olika riktningar, till exempel trä parallellt och vinkelrätt fiberriktning

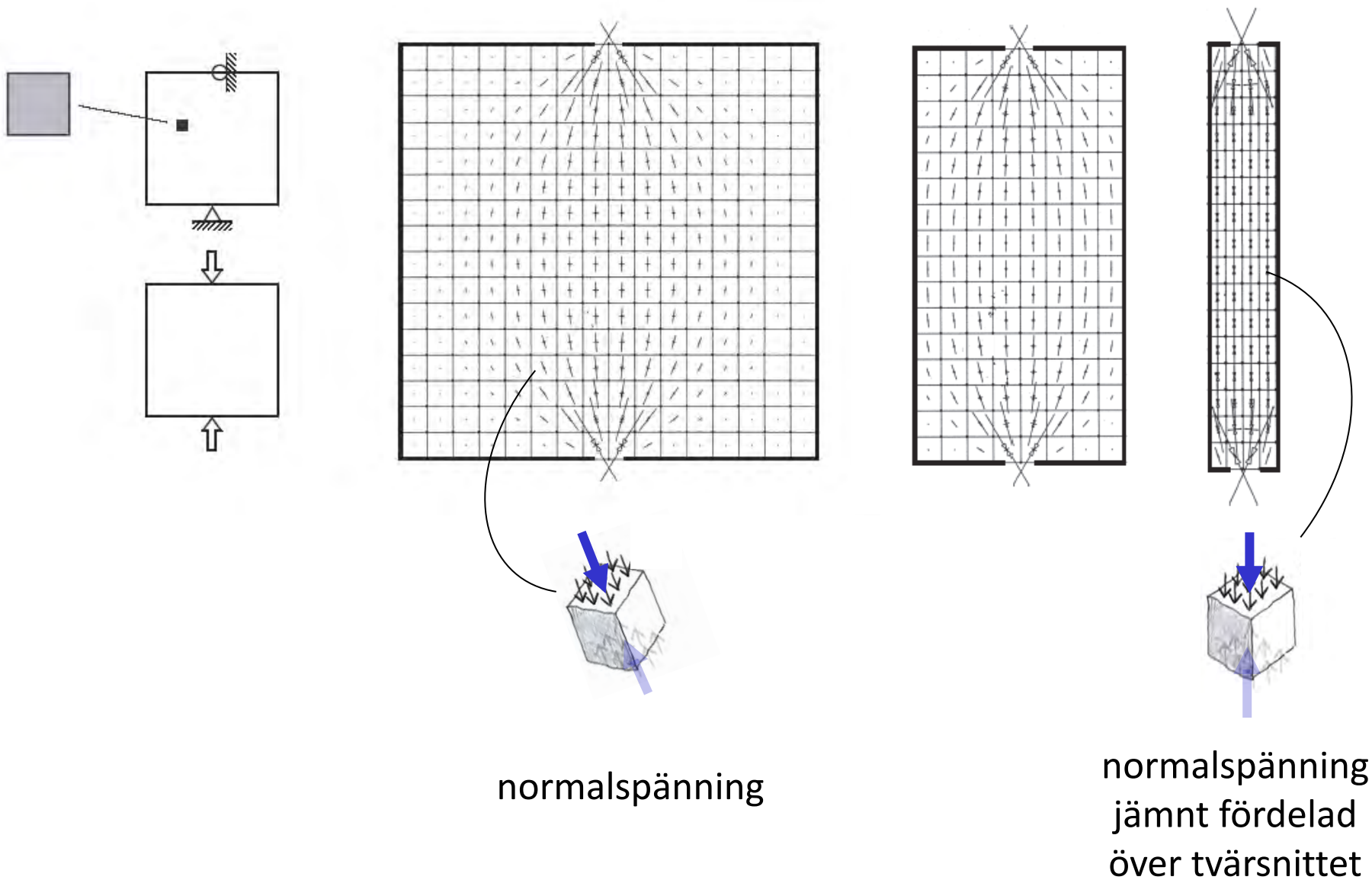
materialstyvhet



materialstyvhet



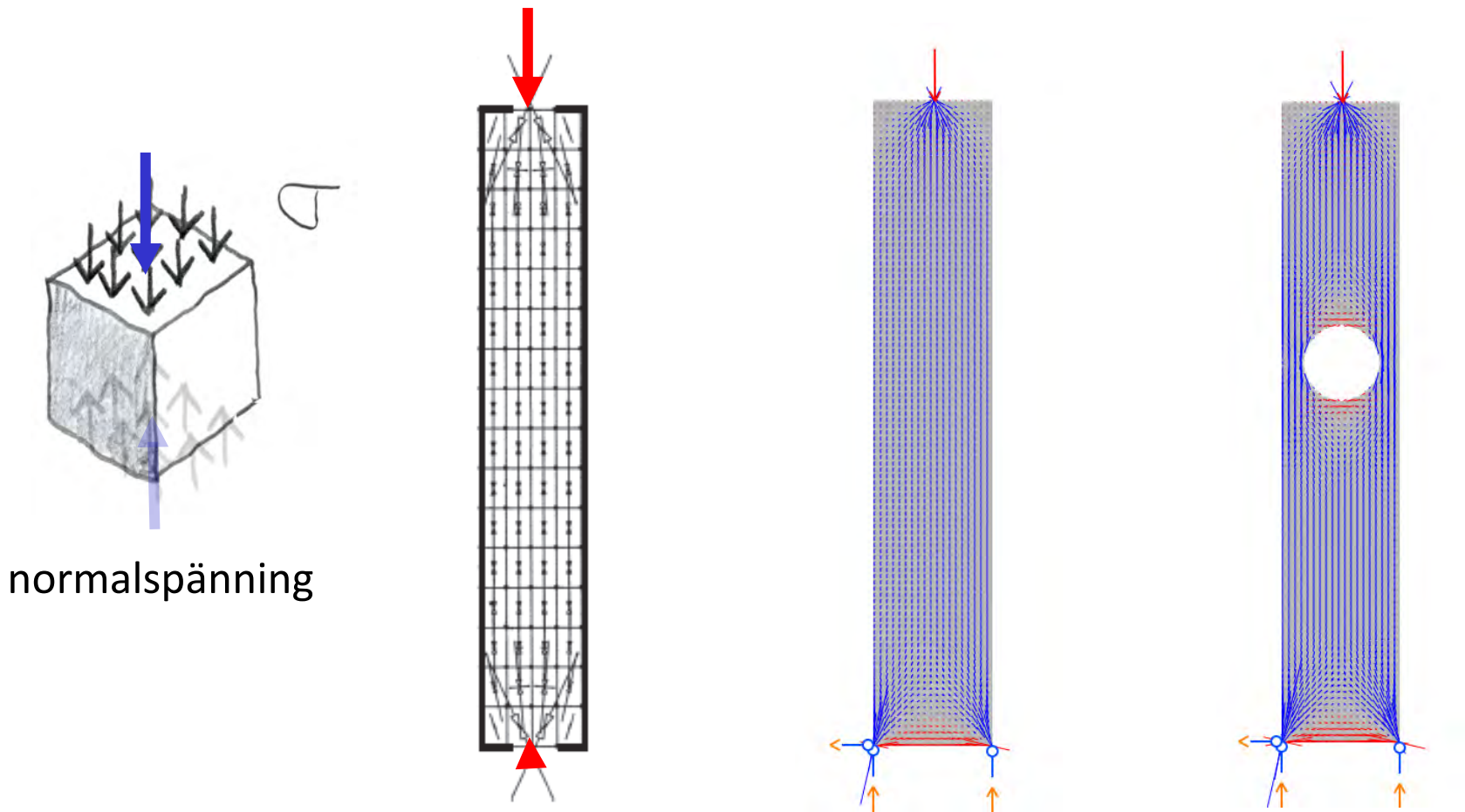
Skivan sedd som en pelare



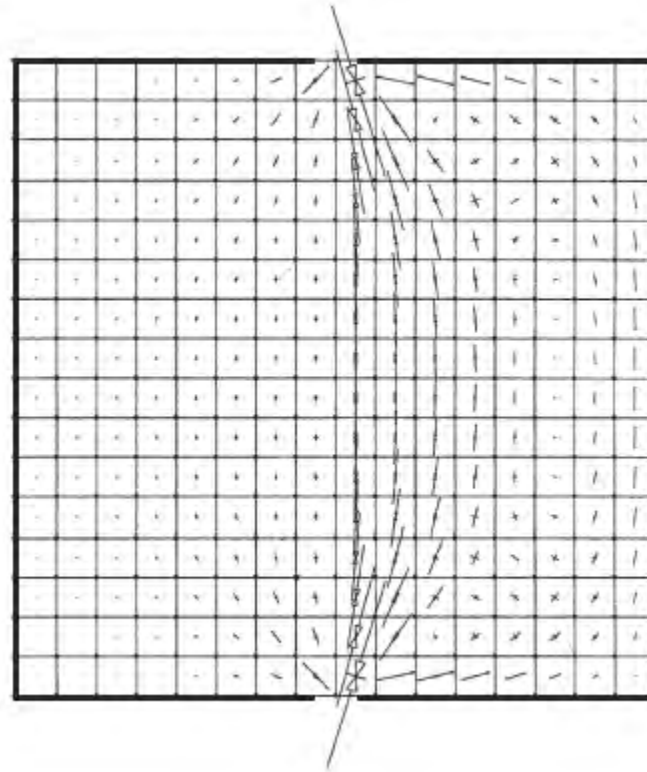
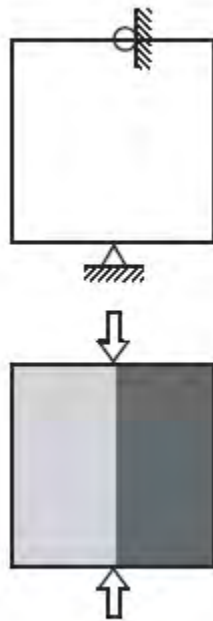
Normalspänningen är inte jämnt fördelad över tvärsnittet

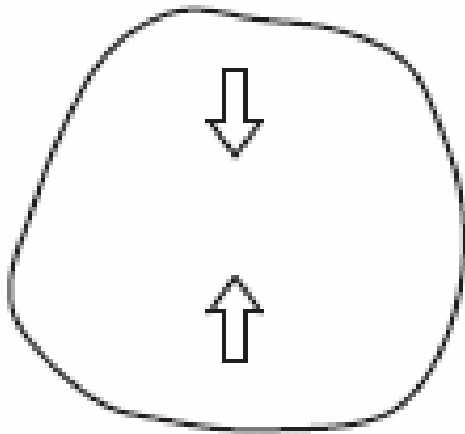
—

vid yttre last, vid stöd och där geometri och materialegenskaper avviker

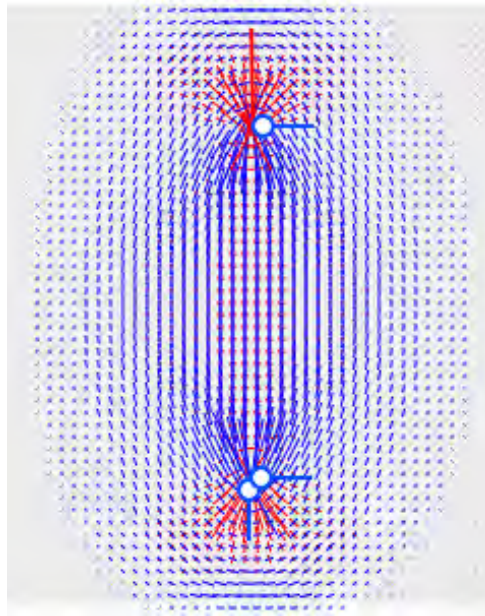


Olika materialstyvhet E i olika delar av skivan

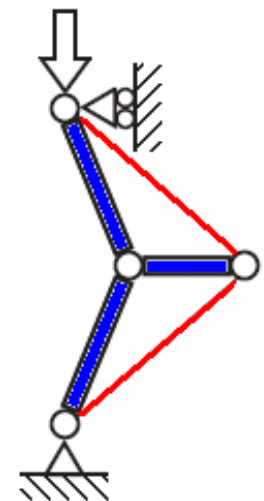
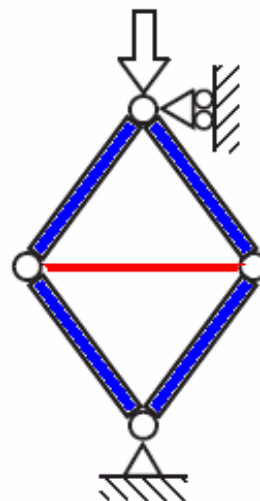
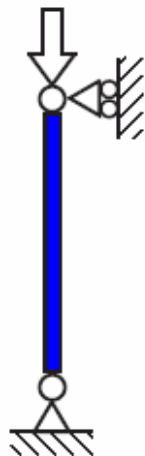
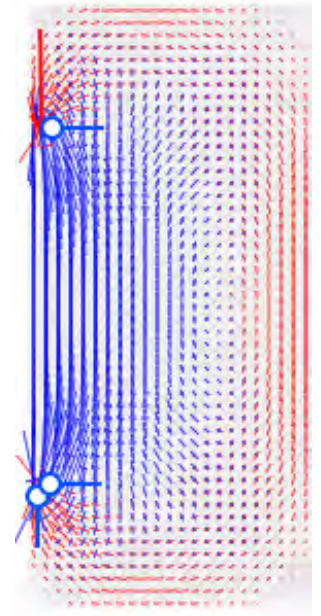




Structural task

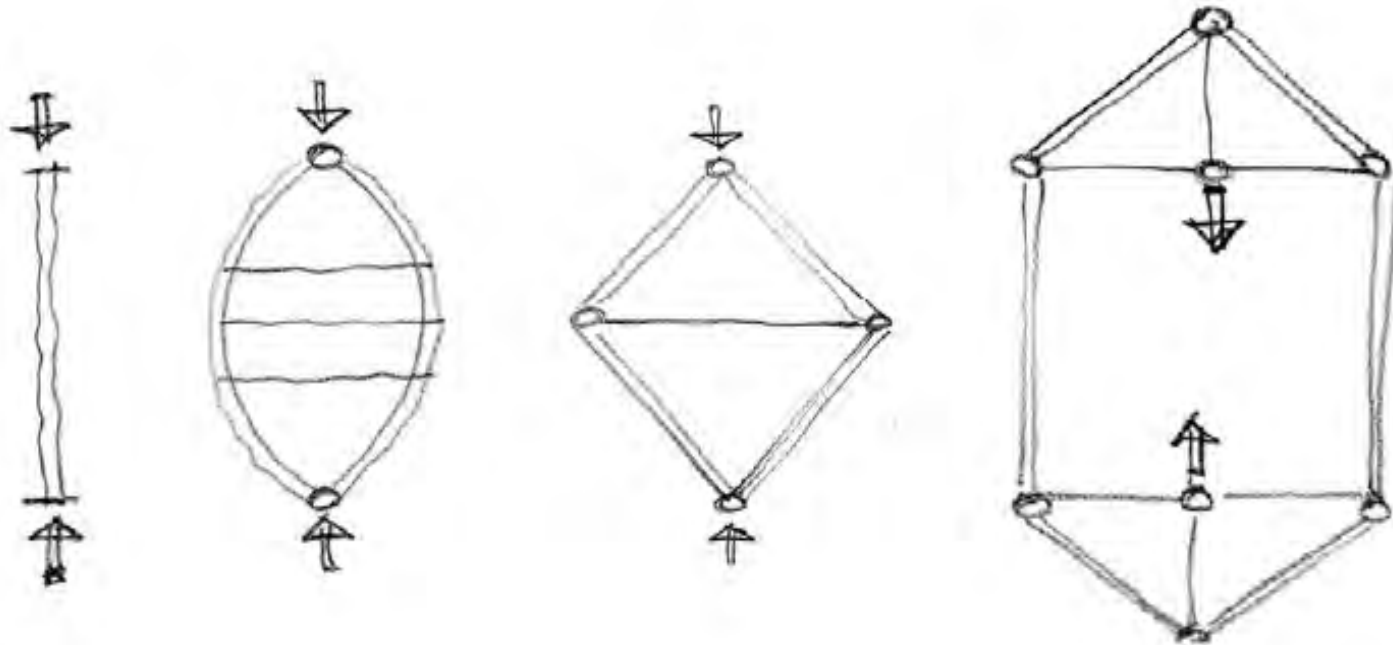


Structural patterns



Potential structures

Vad är en "pelare"?



Effektiva strukturer den i meningen
att de är utformade för att renodlat bära i tryck och drag

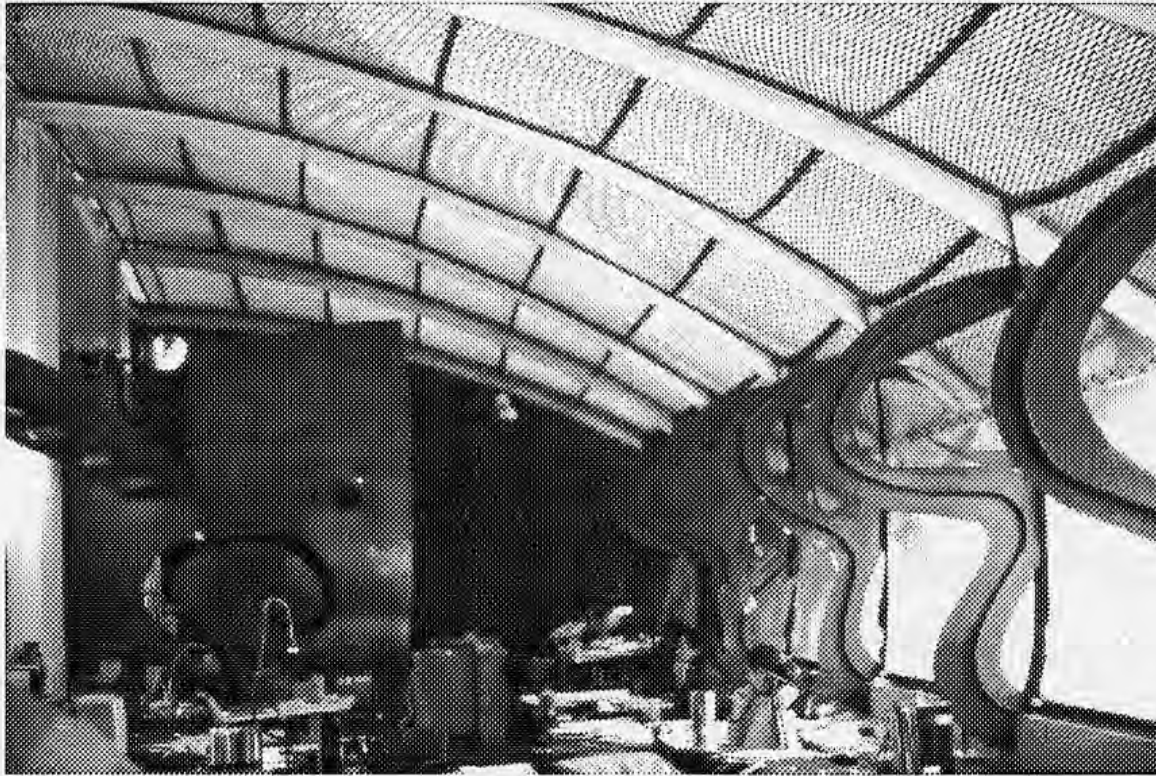
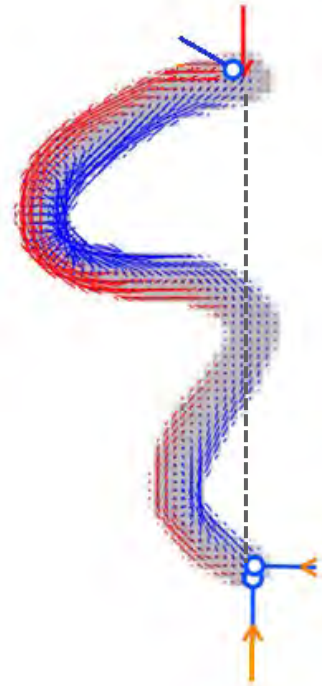


Fig. 3.1 Ron Arad and Alison Brooks; the “One-Off” Studio and Showroom.



Pelare som i huvudsak bär genom böjning
Storleken på böjningen är proportionell mot avståndet
till linjen mellan belastning och stöd

1. Lyfta upp, spänna över, nå ut

VIKTIG UPPGIFT I KURSEN!

Kan vara ett tema för hela läroboken

Gör ett collage med egna skisser/fotografier av bärande strukturer.
Ge först ett ord/ett begrepp som berättar om vad konstruktionen ska bidra till att uttrycka.

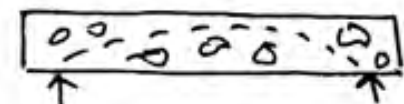
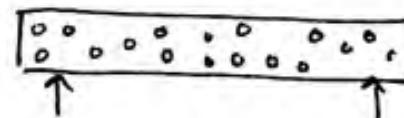
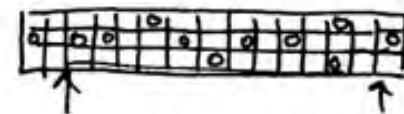
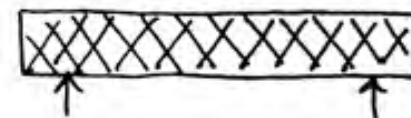
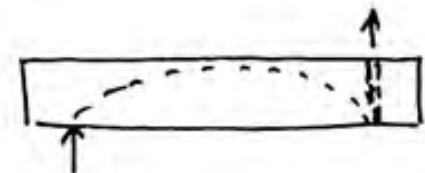
Skissa sedan svarta konturer som visar form, blåa linjer som visar tryck och röda linjer som visar drag.

LYFTA UPP – skissa egna koncept eller fotografera en eller flera ovanliga pelare och väggar

SPÄNNA ÖVER – skissa egna koncept eller fotografera en eller flera fritt upplagda balkar och/eller skivor

NÅ UT – skissa egna koncept eller fotografera en eller flera konsolbalkar/skivor och/eller horisontellt stabiliserande väggar

En träning i att skissa i gränslandet mellan form och kraftmönster



(2. Modellkammaren)

Kan också vara ett tema för hela läroboken

Välj i varje seminariegrupp ut en modell från modellkammaren.

Förklara *den bärande strukturen* genom att förklara:
hur den bär vertikal last
hur den bär horisontell last
(föreläsning 1)

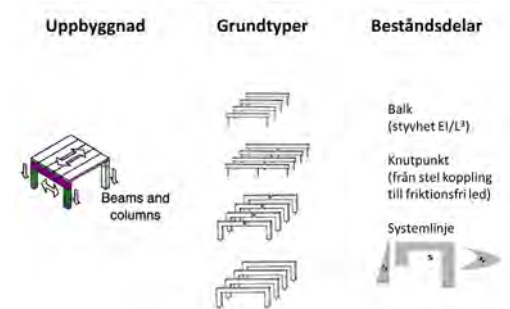
Välj ut minst *ett bärande element* och beskriv detta:
Vilken typ av bärverk det är (om det kan inordnas under någon typ)
Vilka kraftmönster som uppkommer när det belastas (vilka stöd finns,
vilken belastning väljer ni att studera, och vad blir det då för
kraftmönster)
Visa där det är relevant på partikeljämvikter som ni kan identifiera.

Mer om teman för läroboken

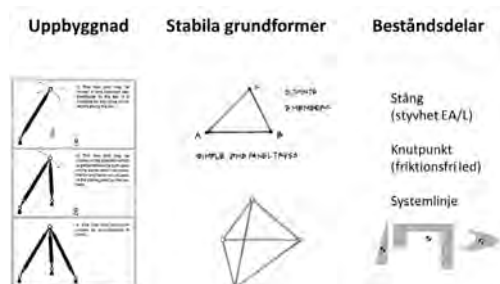
Till exempel **ordlista** som täcker hela kursen

Kedjekurva/trycklinje

Balkar och ramar



Stänger och fackverk



Skivor

Pelare

Pelaren

en stång

i tryck





Nîmes
Foster+Partners

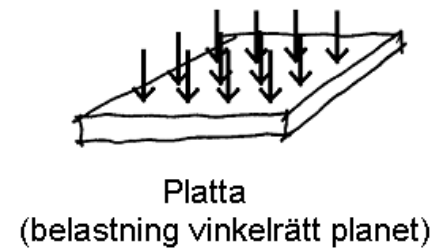
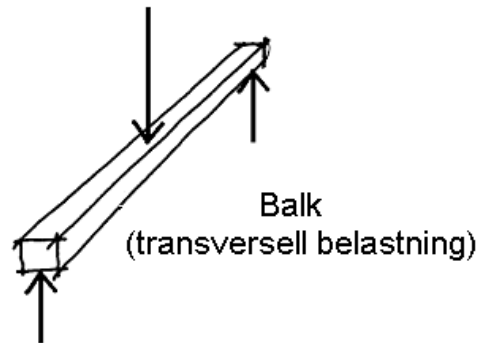
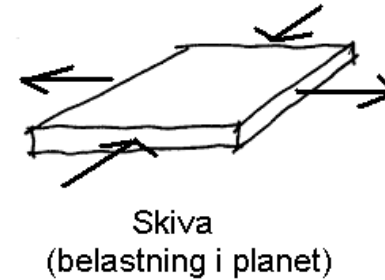
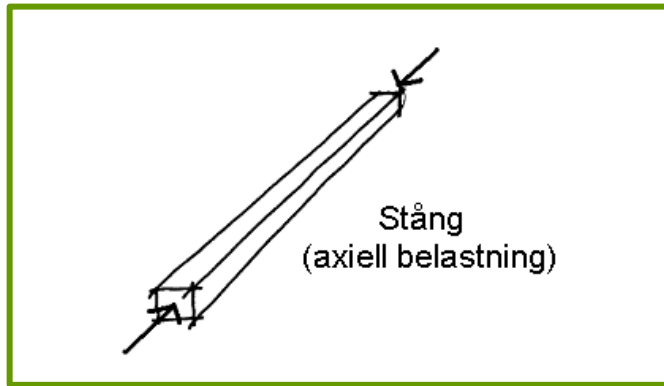


Turin

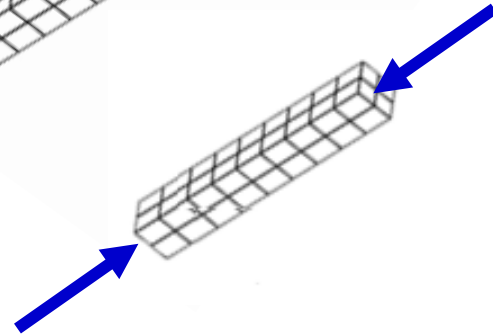
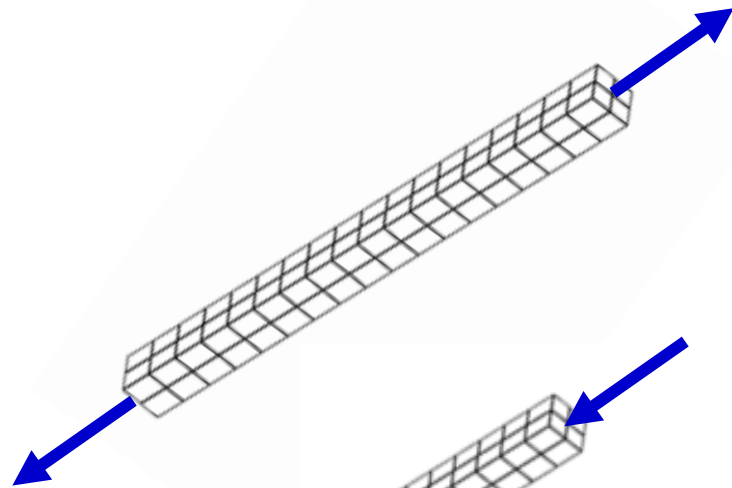


Kolding

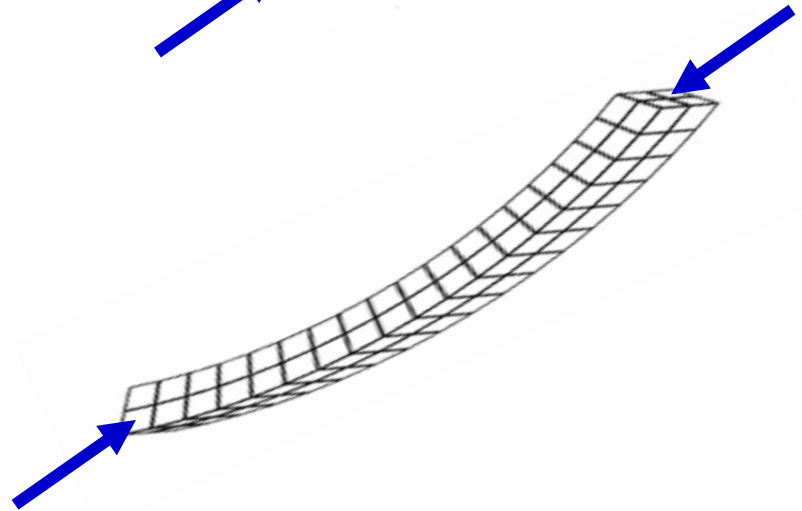
Konstruktionselementens verkningssätt (form + belastning)



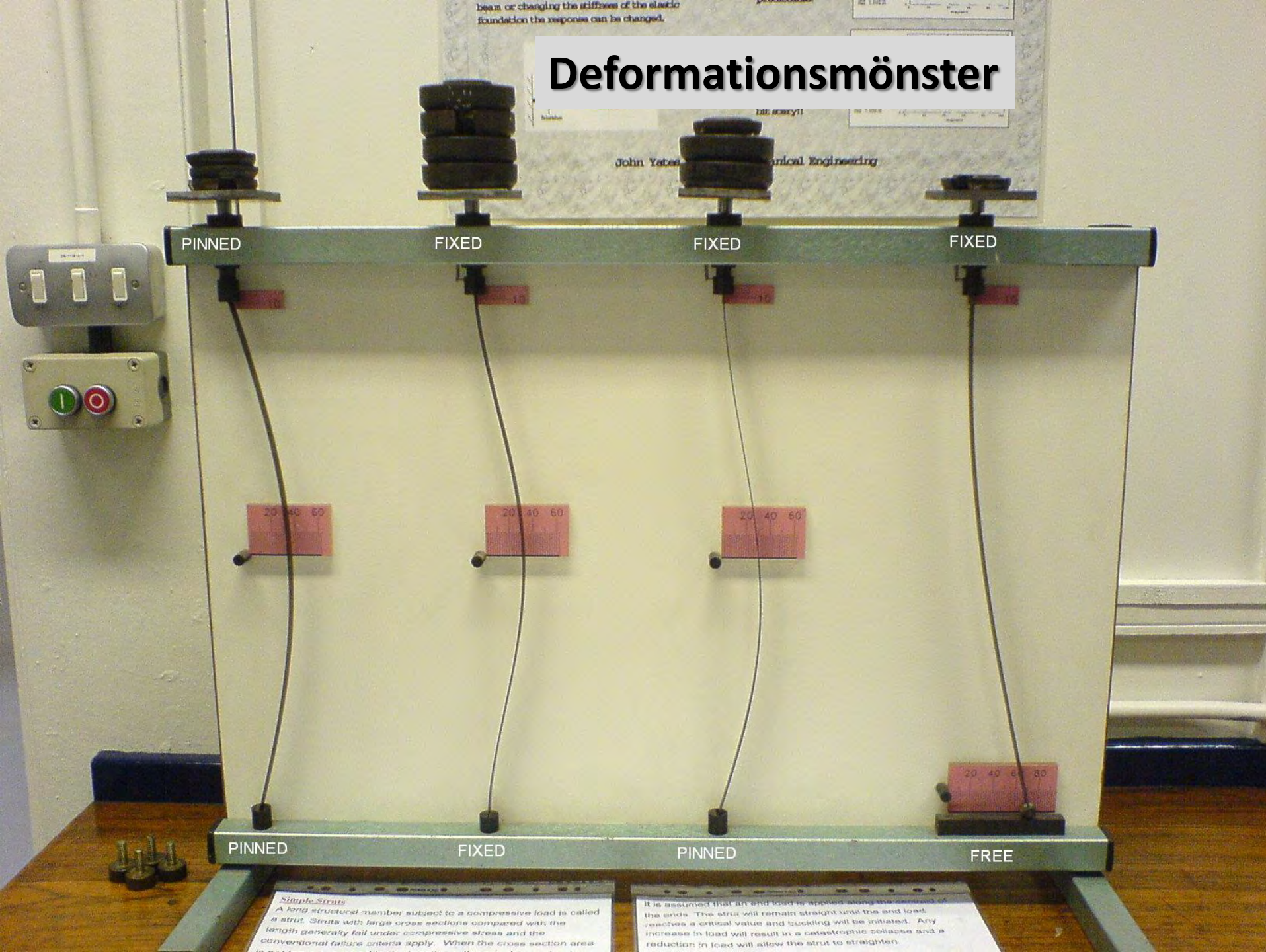
**Dragna och
korta tryckta element**
STÅNGVERKAN



**Långa "slanka"
tryckta element**
BALKVERKAN!



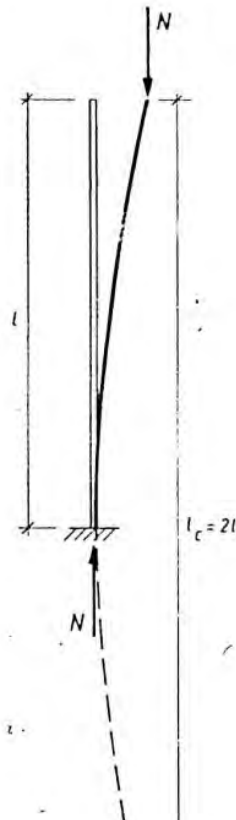
Deformationsmuster



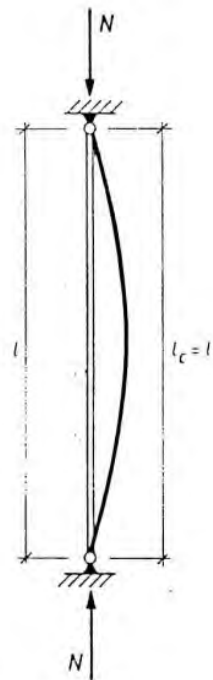
Pelares typologi

ges av **deformationsmönster**
som skapas av **infästningar**
och kallas **Eulers knäckningsfall**

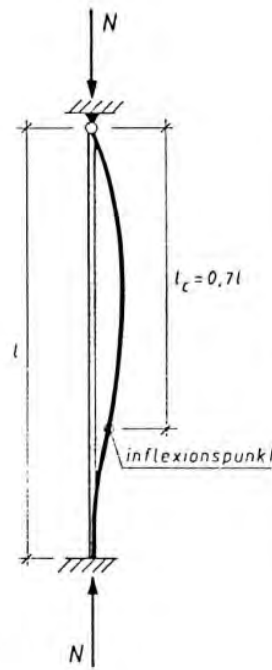
Stång fast inspänd i ena änden, fri i den andra



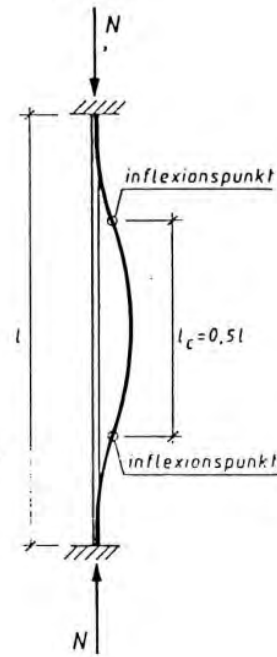
Stång ledad i båda ändar



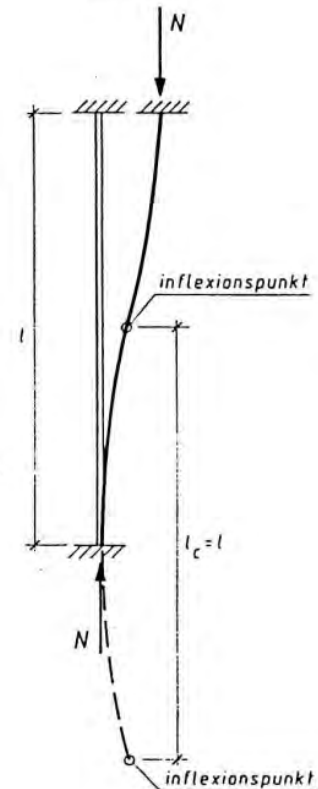
Stång fast inspänd i ena änden, ledad i den andra



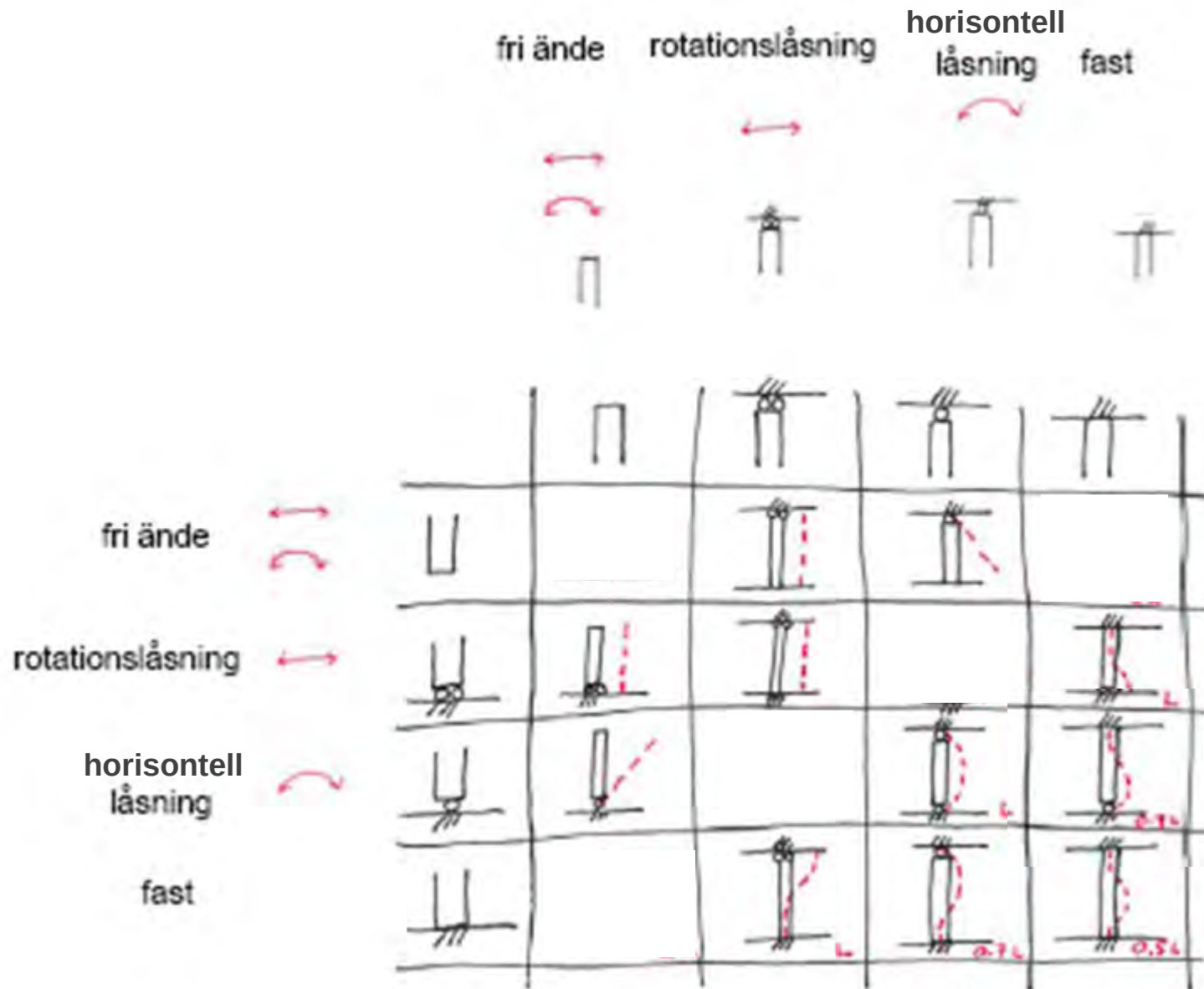
Stång fast inspänd i båda ändar. Ej förskjutbara upplag i sidled



Stång fast inspänd i båda ändar. Upplagert förskjutbara i sidled



Tabell med **pelartyper** sorterade efter infästningar



2. Pelartyper

Gör en tabell som illustrerar olika typer av pelare:

Utgå från pelarens rörelsemöjligheter i sin övre och i sin nedre ände:

- fri horisontell rörelse + fri rotation
- fri horisontell rörelse + låst rotation
- låst horisontell rörelse + fri rotation
- låst horisontell rörelse + låst rotation

Visa för varje pelare det *deformationsmönster* som uppstår.

Observera att flera kombinationer ger pelare som är *mekanismer* (stelkroppsrörelse utan deformation)

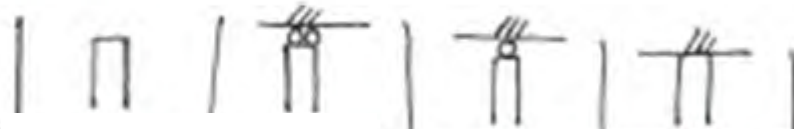
horisontell

fri ände

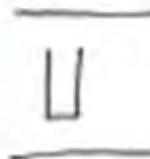
rotationslåsning

låsning

fast



fri ände



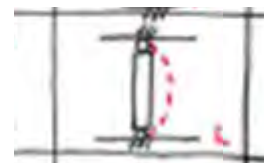
rotationslåsning



horisontell
låsning



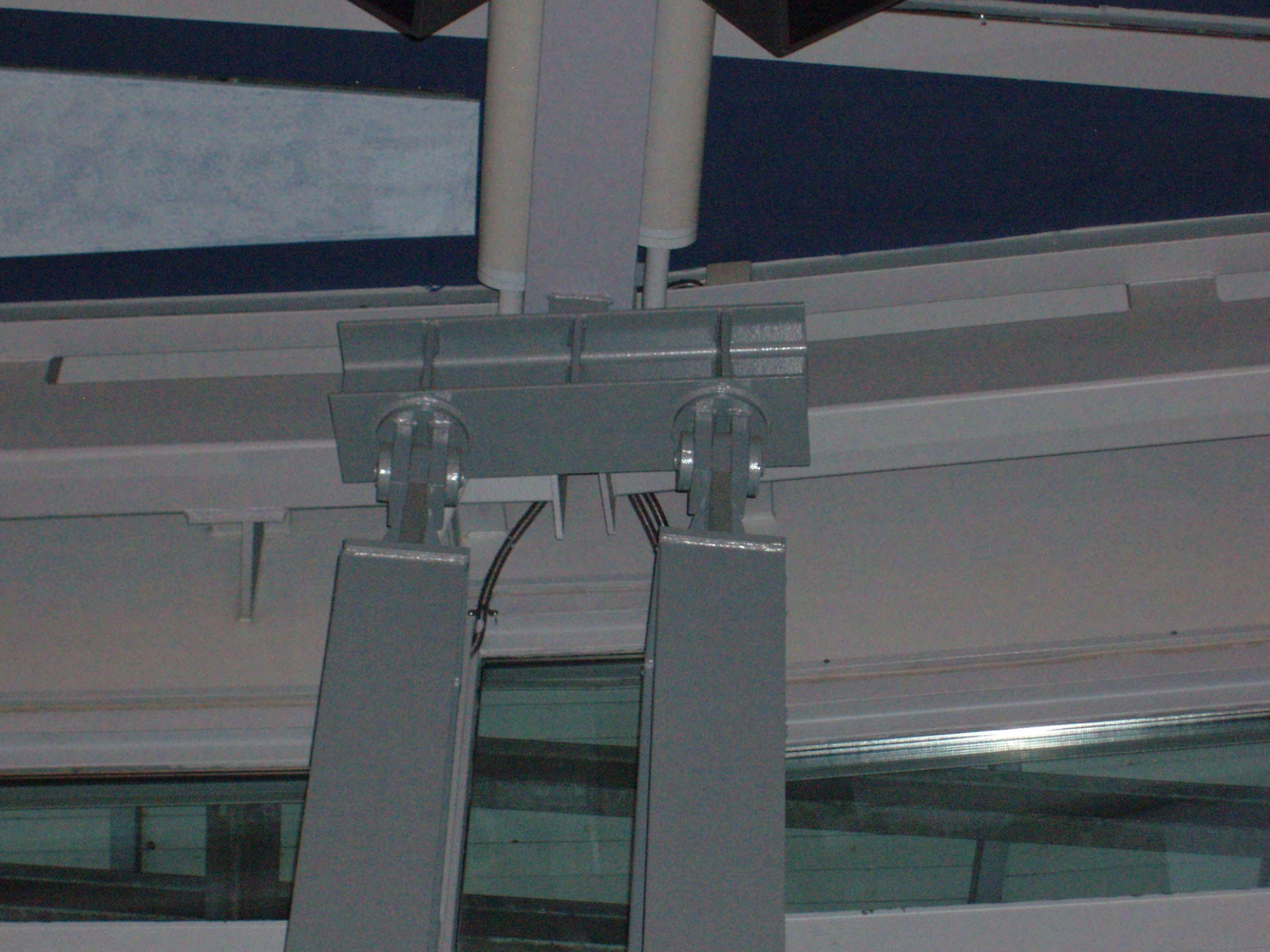
fast



Jan Izikowitz
Göteborgsoperan







horisontell

fri ände

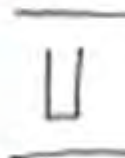
rotationslåsning

låsning

fast



fri ände



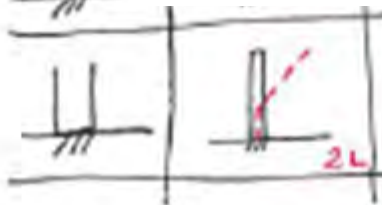
rotationslåsning



horisontell
låsning



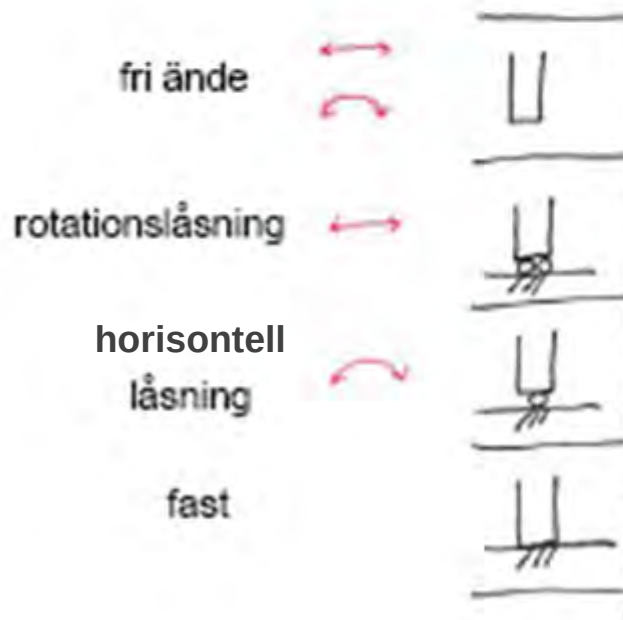
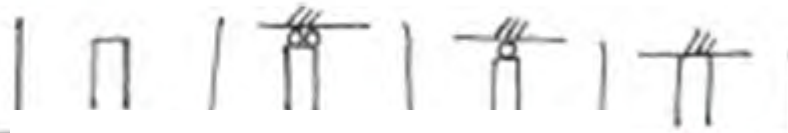
fast



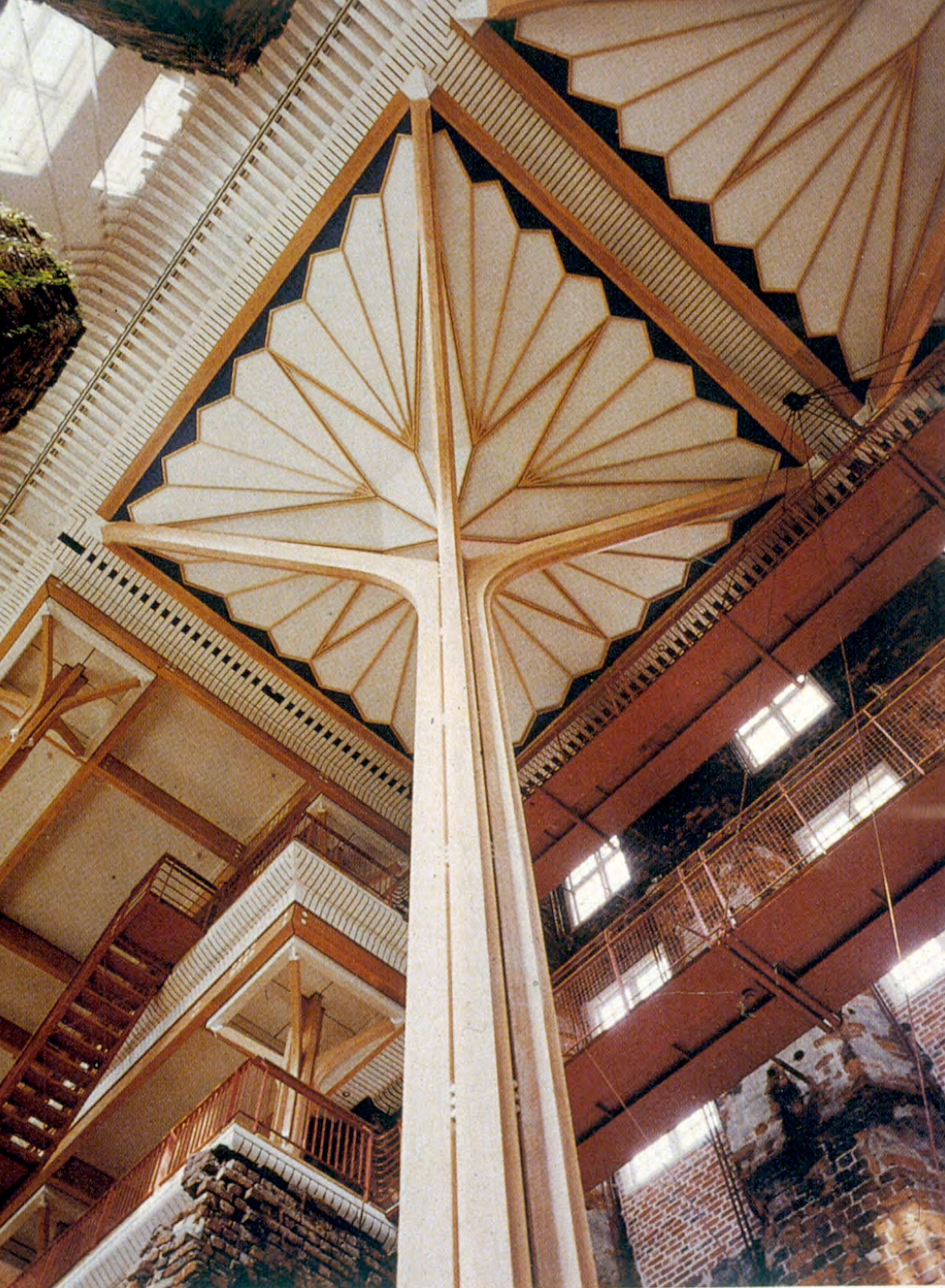


Piero Luigi Nervi
Palazzo del Lavoro
Turin

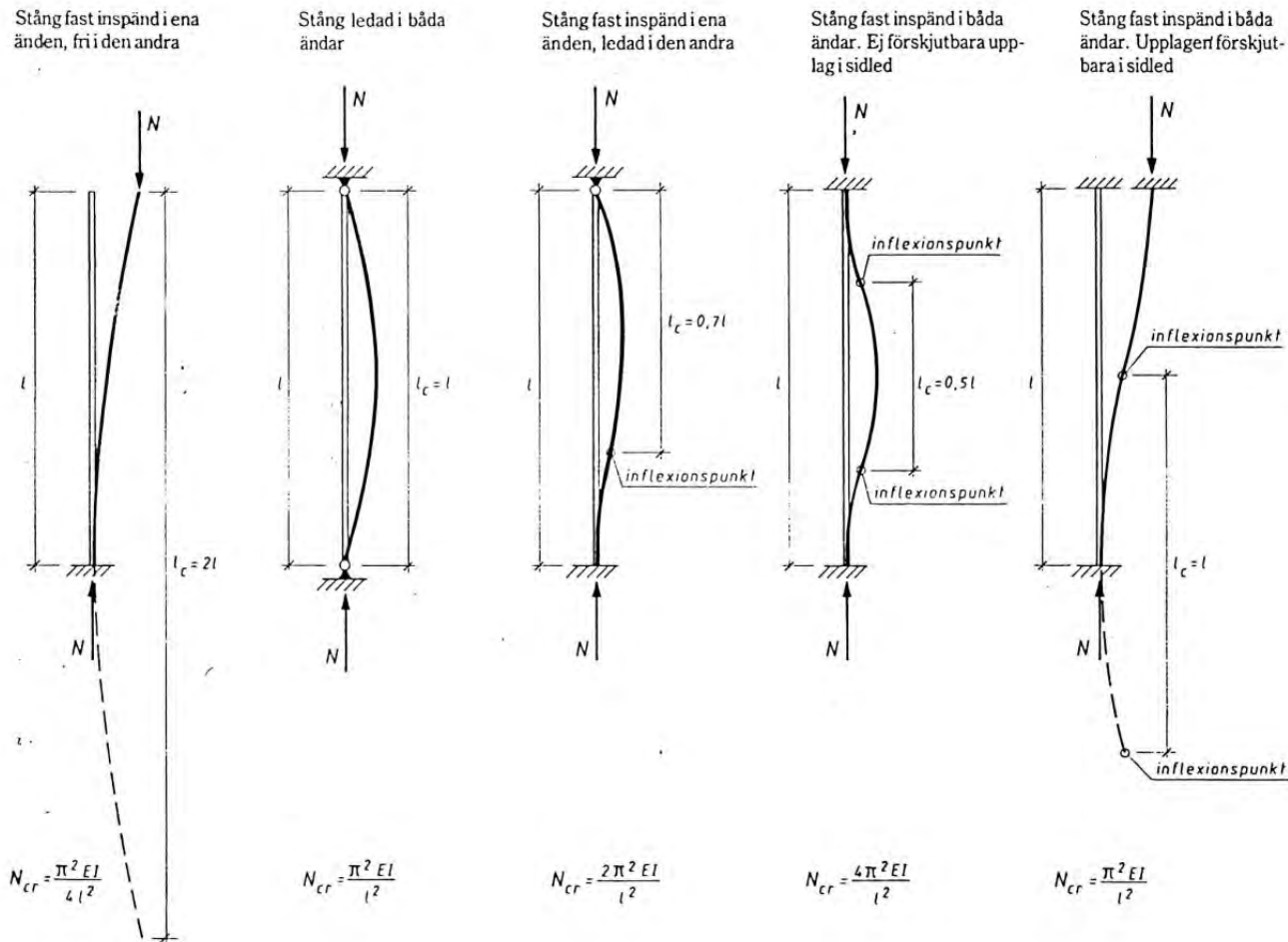
fri ände rotationslåsning horisontell låsning fast



Koldingshus Koldong Jylland

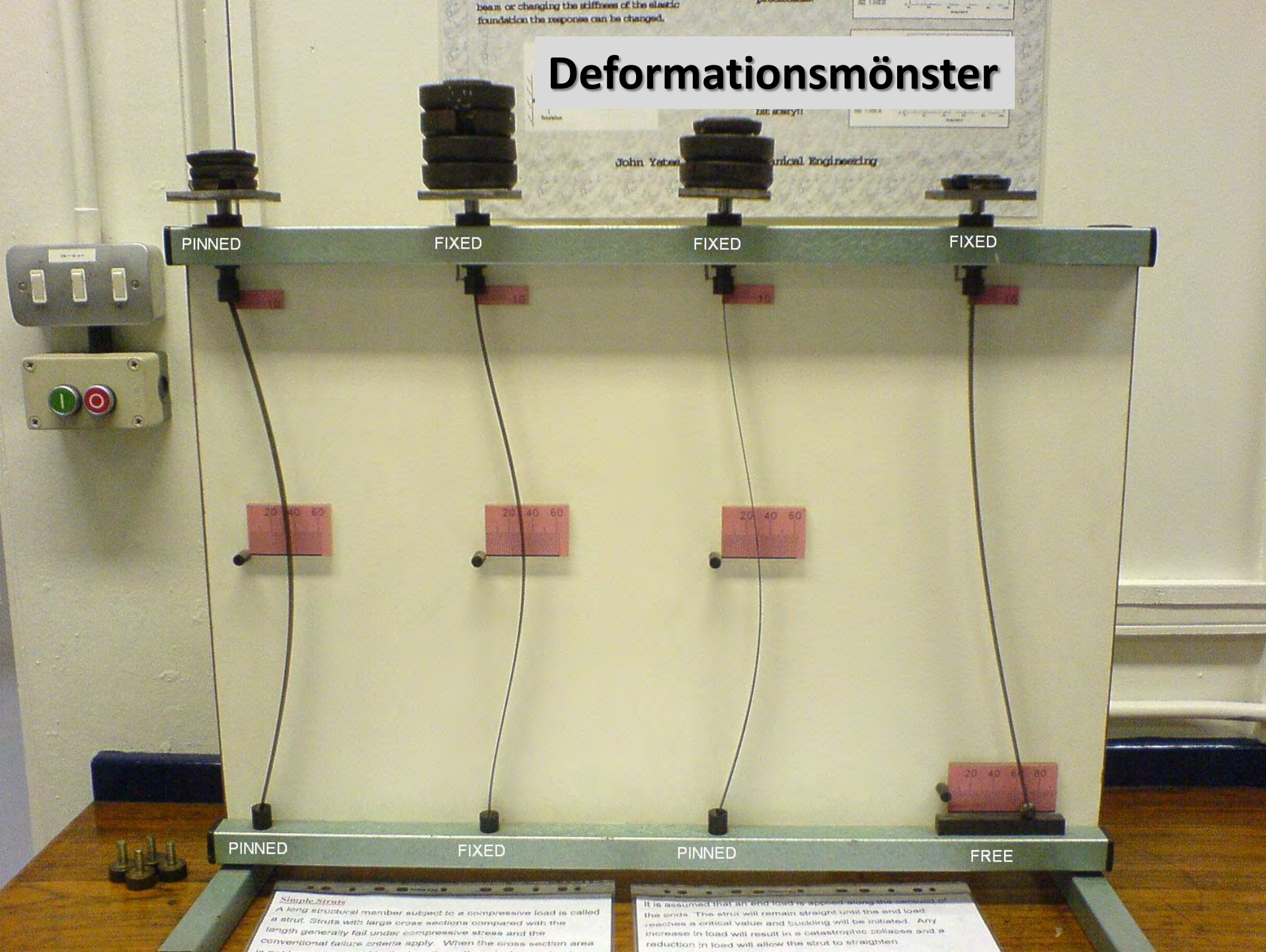


Inflexionspunkt och knäckningslängd

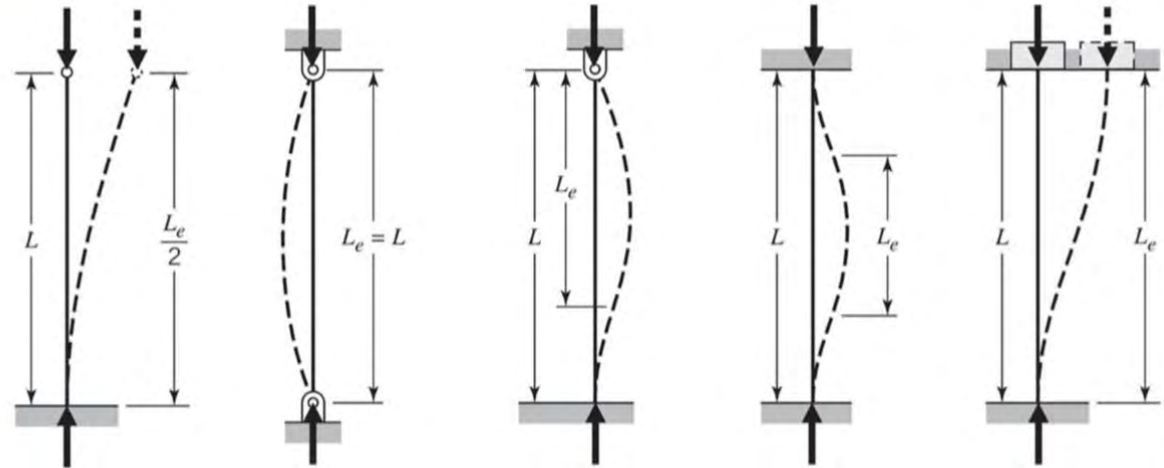
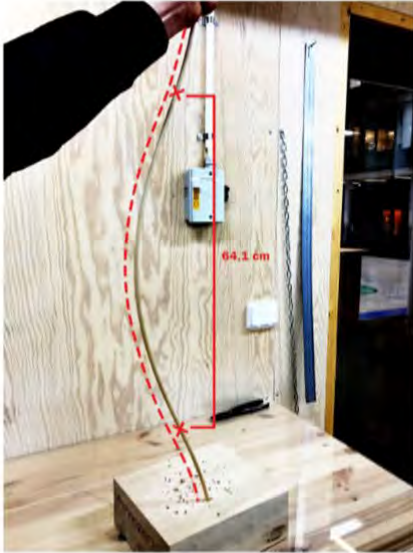


Knäckningslängd l_c är avståndet mellan *inflexionspunkter* (friktionsfria leder kan också ses som inflexionspunkter)

Deformationsmönster



3. Deformationsmönster och knäckningslängd

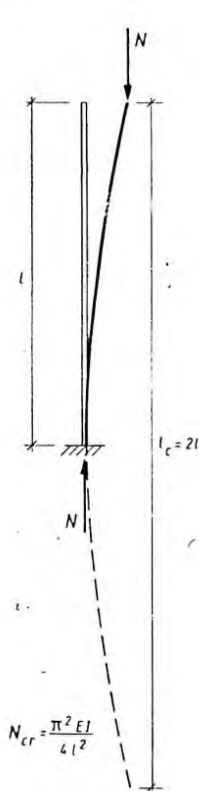


Skaffa en träpinne, 1 m lång och med en tvärsnittsytta på $5 \times 5 \text{ mm}^2$. Pröva att belasta träpinnen med olika inspänningsförhållanden. Känn efter hur styvheten (motståndet mot belastning) påverkas. Dokumentera deformationsfigurerna (fotografera gärna).

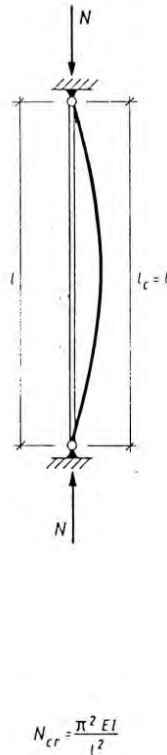
Ange i tabellen i Uppgift 2 *knäckningslängden* för respektive pelare.

Eulers knäckningsformel

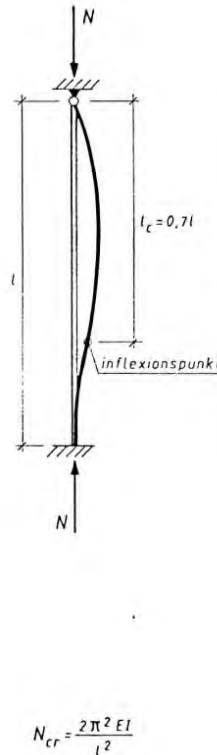
Stång fast inspänd i ena änden, fri i den andra



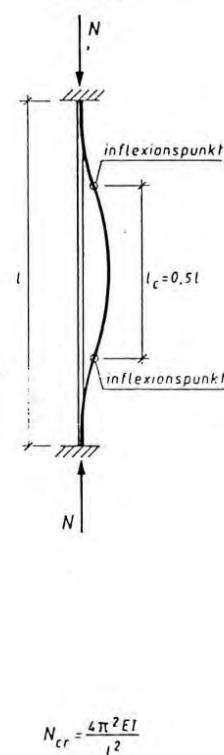
Stång ledad i båda ändar



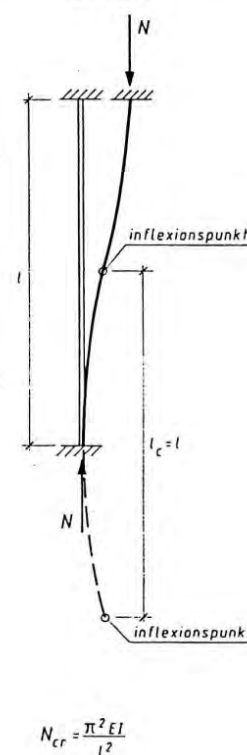
Stång fast inspänd i ena änden, ledad i den andra



Stång fast inspänd i båda ändar. Ej förskjutbara upplag i sidled



Stång fast inspänd i båda ändar. Upplaget förskjutbara i sidled

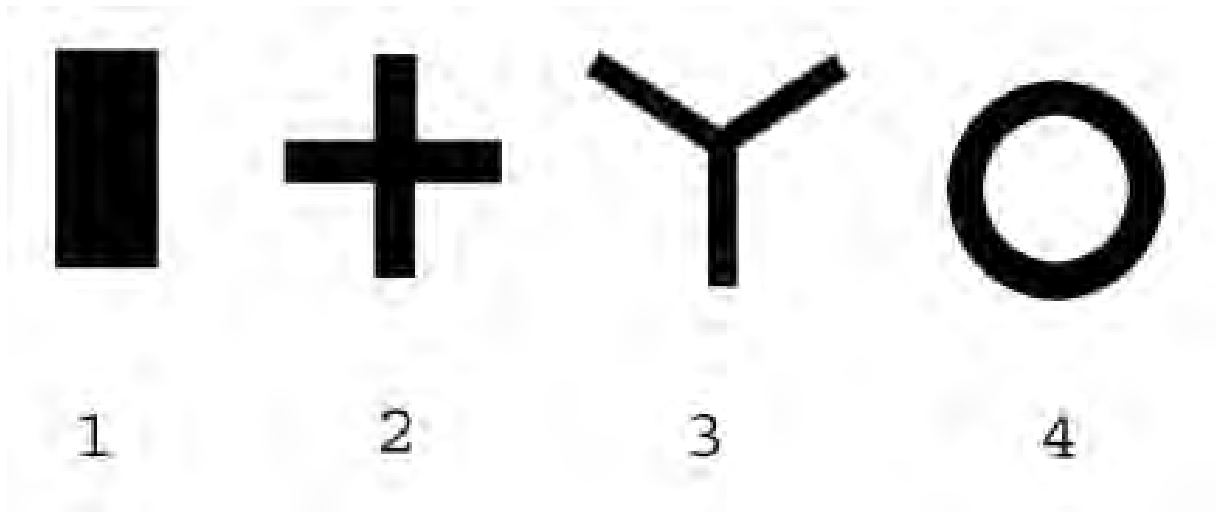


Eulers knäckningsformel

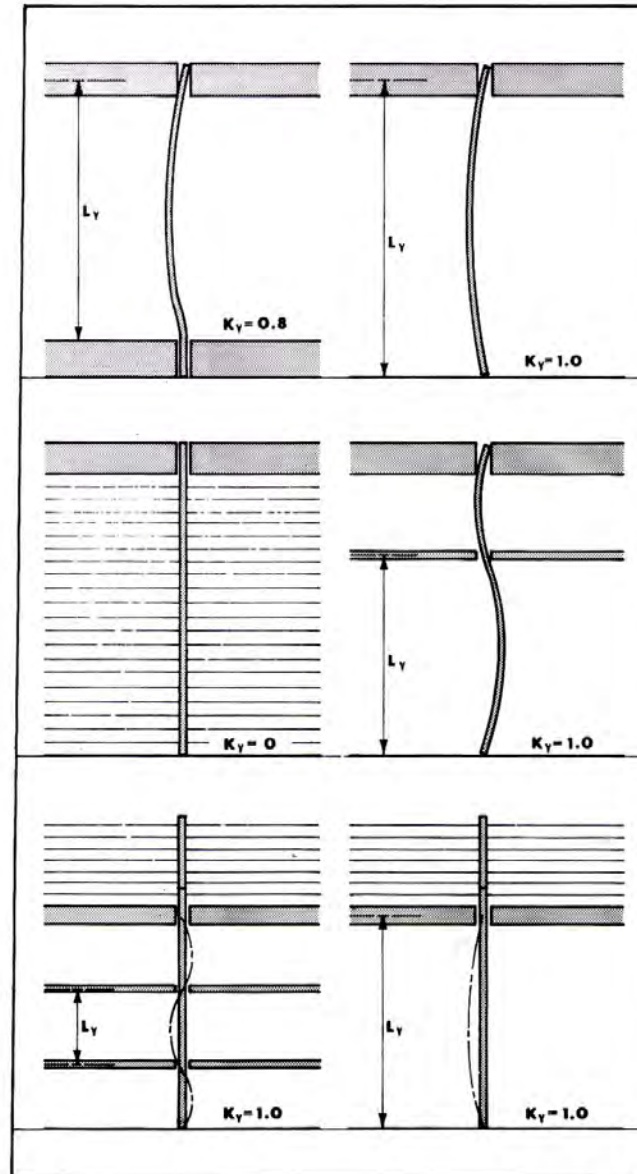
$$N_{cr} = \pi^2 EI / L_c^2$$

Bärförmåga skapas av
längd, *tvärsnitt* och *material*
(jfr böjstyvhet EI/L^3)

Utknäckning kan ske i alla riktningar



Knep för att minska knäckningslängden





4. Knäckningslängd och tvärsnittsform

$$N_{cr} = \pi^2 EI / L_c^2$$

Ett tryckt element knäcks alltid ut i sin svagaste riktning. Det kan vara den riktning som har den *längsta knäckningslängden* eller den riktning som har det *minsta tröghetsmomentet*.

Visa exempel från arkitekturen där man åstadkommit styva och starka pelare, genom att kombinera ett styvt tvärsnitt i en riktning med en kort knäckningslängd i den andra riktningen.

Macro Contemporary Art Museum – Rome

DRAG – ingen risk för knäckning



ODBC
Odile Decq
Benoit Cornette